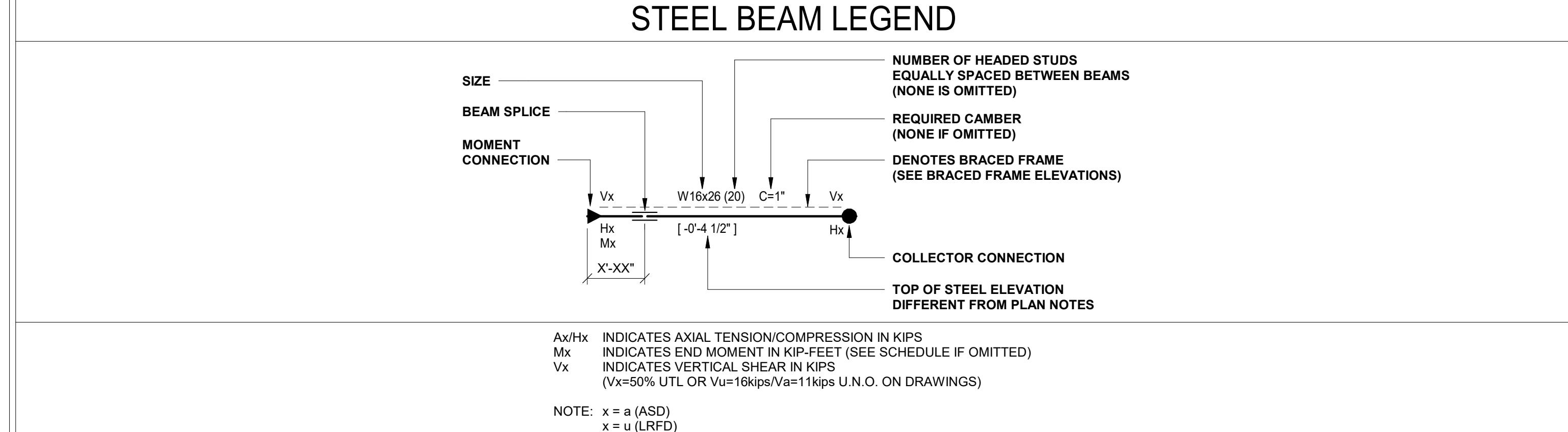


PLAN SYMBOLS LEGEND



PLAN GRAPHICS LEGEND

MATERIAL LEGEND

PLAN SYMBOLS LEGEND

DESIGN CRITERIA

DESIGN CRITERIA

- RISK CATEGORY: II
- DEAD LOAD:
ROOF: 20 PSF
MEP/CEILING/DECK/MISC 250 LBS
CUH1 250 LBS
CUH2 250 LBS
- LIVE LOAD: MAIN ROOF 20 PSF
- SNOW LOAD: TERRAIN CATEGORY C
IMPORTANCE FACTOR I=1.0
THERMAL FACTOR C=1.0
EXPOSURE FACTOR C=1.0
GROUND SNOW LOAD P=20 PSF
ROOF SNOW LOAD P=20 PSF
- WIND LOAD: MAIN BUILDING: BASIC WIND SPEED V=108 MPH(FACTORED)
IMPORTANCE FACTOR I=1.0
EXPOSURE CATEGORY C
TOPOGRAPHIC FACTOR Ks = 1.0
WIND DIRECTIONALITY FACTOR GCp = ±0.18
- SEISMIC LOAD: IMPORTANCE FACTOR I=1.0
MAPPED SPECTRAL RESPONSE ACCELERATIONS S=0.394
SHORT PERIOD PARAMETER S=0.154
1 SECOND PARAMETER C
SOIL SITE CLASS DESIGN SPECTRAL RESPONSE ACCELERATIONS S=0.342
SHORT PERIOD PARAMETER S=0.154
1 SECOND PARAMETER C
SEISMIC DESIGN CATEGORY C
ANALYSIS METHOD: EQUIVALENT LATERAL FORCE PROCEDURE
BASIC SEISMIC-FORCE-RESISTING SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED
MOMENT FRAMES
RESPONSE MODIFICATION FACTOR R=3.5
DEFLECTION AMPLIFICATION FACTOR C=3.0
OVER STRENGTH FACTOR Ω=3.0
SEISMIC RESPONSE COEFFICIENT C=0.098
- FOUNDATIONS: ALLOWABLE BEARING PRESSURE 2500 PSF
SPREAD FOOTINGS 2000 PSF
CONTINUOUS FOOTINGS FROST DEPTH 30"

CODES AND STANDARDS

CODES AND STANDARDS (LATEST EDITION, U.N.O.)

- PROJECT BUILDING CODE: IBC 2018
- DESIGN LOADS:
A. ASCE 7-16
- CONCRETE CONSTRUCTION:
A. ACI 301
B. ACI 304
C. ACI 305
D. ACI 306
E. ACI 308
F. ACI 309
G. ACI 315
H. ACI 318
I. ACI 347
- STEEL CONSTRUCTION:
A. AISC 360
B. AISC 341
C. AISC 358
D. AWS
E. SJI COSP
F. SDI COSP
- COLD-FORMED METAL FRAMING:
A. AISI
B. SSMA
- MASONRY CONSTRUCTION:
A. TMS 402/602
- WOOD CONSTRUCTION:
A. NDS

ABBREVIATION	DEFINITION	ABBREVIATION	DEFINITION
AB	ANCHOR BOLTS	F, FTG	FOOTING
ADDNL	ADDITIONAL	GB	GRADE BEAM
AFF	ABOVE FINISH FLOOR	HGT	HEIGHT
ALT	ALTERNATIVE	HK	HOOK
ARCH	ARCHITECT	HORIZ	HORIZONTAL
ARCHT	ARCHITECTURAL	IF	INNER FACE
B, BOTT	BOTTOM	INT	INTERIOR
BB	BOND BEAM	JB	JOIST BEARING
BS	BRICK SHELF/EDGE	J	JEDGE
BLDG	BUILDING	L	LATERAL
BM	BEAM	LAT	LONG LEG HORIZONTAL
BMD	BOTTOM OF METAL DECK	LLH	LONG LEG VERTICAL
BRG	BEARING	LLV	LONGITUDINAL
CC	CENTER TO CENTER	LONG	MASONRY
CJ	CONTROL JOINT	MAS	MAXIMUM
CL	CENTERLINE	MAX	MECHANICAL
CLR	CLEAR	MECH	MANUFACTURER
CMU	CONCRETE MASONRY UNIT	MFR	MINIMUM
COL	COLUMN	MIN	METAL
C, CONC	CONCRETE	MTL	NOMINAL CONTRACT
CONN, CONNX	CONNECTION	NIC	NON
CONST	CONSTRUCTION	NOM	NEAR SIDE
CONT	CONTINUOUS	NS	OUTER FACE
DET, DTL	DETAIL	OF	OPPOSITE HAND
DIM	DIMENSION	OH	OPENING
DK	DECK	OPNG	POST ABOVE
DS	DIAGONAL SHEATHING	PA	PRECAST
DWGS	DRAWINGS	PC	PLATE
DWL	DOWELS	PL	REINFORCING
EA	EACH	REINF	REQUIRED
EE	EXTENDED END	REQD	RETAINING
EF	EACH FACE	RET	SC
EFF	EFFECTIVE	SCHED	SECCION
EU	EXPANSION JOINT	SCOT	SECTION
EL, ELEV	ELEVATION	SPAC	SPACING
EF	EACH FACE	STIFF	STIFFENER
EOS	EDGE OF SLAB	STL	STEEL
ES	EACH SIDE	T	TOP
EW	EACH WAY	Txx	TOP OF XX
EXIST	EXISTING	TRAN	TRANSVERSE
EXP	EXPANSION	TYP	TYPICAL
EXT	EXTERIOR, EXTENSION	UNO	UNLESS NOTED OTHERWISE
FF	FINISH FLOOR	VERT	VERTICAL
FL	FLOOR	W	WIDE, WIDTH
FS	FAR SIDE	WWF	WELDED WIRE FABRIC

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REPRODUCTION OF THIS DRAWING FOR SHOP DRAWINGS
REMOVED ALL SEALS, TRIM, NAMES, TITLES, SHEET NUMBERS
AND SMALL SCALE, FULL RESPONSIBILITY FOR THEIR
EXHIBITING IS WITH THE ENGINEER.

THE PROFESSIONAL ENGINEER'S SEAL APPLIED TO THIS SHEET INDICATES THAT THE PREPARATION OF THE DRAWING WAS DIRECTED BY THE PROFESSIONAL ENGINEER WHO IS RESPONSIBLE FOR THE DRAWING.
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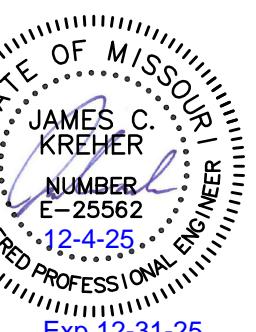
STATE OF MISSOURI
JAMES C. KREHER
NUMBER E-25562
PROFESSIONAL ENGINEER
Exp 12-31-25

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Revisions:

SHEET LIST		
SHEET NO.	SHEET NAME	ISSUE DATE
S0.0	LEGENDS AND SYMBOLS	12/04/25
S0.1	GENERAL NOTES	12/04/25
S0.11	GENERAL NOTES	12/04/25
S0.12	SPECIAL INSPECTIONS	12/04/25
S0.2	CONCRETE TYPICAL DETAILS	12/04/25
S0.4	FRAMING TYPICAL DETAILS	12/04/25
S1.0	FOUNDATION PLAN	12/04/25
S1.01	TRASH ENCLOSURE PLAN	12/04/25
S1.1	LOW ROOF FRAMING PLAN	12/04/25
S1.2	CLERESTORY ROOF FRAMING PLAN	12/04/25
S2.0	FOUNDATION DETAILS AND SECTIONS	12/04/25
S4.0	ROOF FRAMING SECTIONS	12/04/25

LEGENDS AND SYMBOLS
S0.0
ISSUE DATE: 12.04.2025
JOB NUMBER: 25-021



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Revisions:

GENERAL NOTES:

THE GENERAL NOTES ARE NOT A SUBSTITUTE OR A REPLACEMENT TO THE PROJECT SPECIFICATIONS. THESE NOTES ARE INTENDED AS A GUIDE TO THE DESIGN AND/OR CONSTRUCTION REQUIREMENTS ESTABLISHED FOR THIS PROJECT. NO CONTRACTOR SHOULD ATTEMPT TO DESIGN, BID OR CONSTRUCT ANY PORTION OF THE WORK HEREIN WITHOUT CONSULTING THE PROJECT SPECIFICATIONS, WHERE CONFLICTS OCCUR BETWEEN THESE NOTES AND THE SPECIFICATIONS THE MORE STRINGENT REQUIREMENTS SHALL GO. LESS STRINGENT REQUIREMENTS RELATED TO THE STRUCTURAL ENGINEER VARIATION IN THE FIELD CONDITIONS RELATED TO THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT. WORK SHALL NOT PROGRESS UNTIL WRITTEN PERMISSION FROM THE ARCHITECT IS OBTAINED.

CONSTRUCTION AND SAFETY:

- THE CONTRACTOR AND THEIR SUBCONTRACTORS ARE SOLELY RESPONSIBLE FOR ALL SAFETY REGULATIONS, PROGRAMS AND PRECAUTIONS RELATED TO ALL WORK ON THIS PROJECT.
- THE CONTRACTOR AND THEIR SUBCONTRACTORS ARE SOLELY RESPONSIBLE FOR THE PROTECTION OF PERSONS AND PROPERTY EITHER ON OR ADJACENT TO THE PROJECT AND SHALL PROTECT SAME AGAINST INJURY, DAMAGE OR LOSS.
- MEANS AND METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIALS ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.
- THE STRUCTURAL DRAWINGS ARE INTENDED TO BE USED IN CONJUNCTION WITH THE DRAWINGS OF OTHER CONSULTANTS AND TRADES. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE VARIOUS REQUIREMENTS.
- THE CONTRACTOR AND THEIR SUBCONTRACTORS ARE RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE DURING DEMOLITION AND OR CONSTRUCTION. SUCH LOADS SHALL NOT EXCEED THE CAPACITY IF THE STRUCTURE AT ANY TIME.
- ALL DEMOLITION AND OR CONSTRUCTION PROCEDURES SHALL BE REVIEWED BY A SPECIALTY CONSTRUCTION ENGINEER. SEE **DEFERRED SUBMITTALS** SECTION OF THE GENERAL NOTES.
- NO CHANGES IN SIZE, DIMENSION OR LOCATION SHALL BE MADE IN ANY STRUCTURAL ELEMENTS WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
- CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH NEW WORK IN AREAS Affected BY EXISTING CONDITIONS. STRUCTURAL ENGINEER SHALL BE INFORMED IN WRITING OF CONFLICTS BETWEEN EXISTING AND PROPOSED NEW CONSTRUCTION.
- CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL DIMENSIONS SHOWN ON THE CONTRACT DOCUMENTS, INCONSISTENCIES ON THE STRUCTURAL DRAWINGS OR BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER CONTRACT, SHOP FABRICATION, OTHER DRAWINGS OR INFORMATION SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH AFFECTED WORK.
- DO NOT SCALE THESE DRAWINGS, USE THE DIMENSION SHOWN.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION AND ANY TEMPORARY BRACING FOR LOADS INDUCED DURING CONSTRUCTION OR SUPPORT REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL INFORM THE STRUCTURAL ENGINEER, CLEARLY AND EXPLICITLY IN WRITING OF ANY DEVIATION OR SUBSTITUTION OF REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF ANY REQUIREMENTS OF THE CONTRACT DOCUMENTS BY VIRTUE OF THE STRUCTURAL ENGINEER'S REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS CLEARLY AND EXPLICITLY INFORMED THE STRUCTURAL ENGINEER IN WRITING OF ANY DEVIATIONS OR SUBSTITUTIONS AT TIME OF SUBMISSION, AND THE STRUCTURAL ENGINEER HAS BEEN GIVEN WRITTEN APPROVAL FOR THE SPECIFIC DEVIATIONS OR SUBSTITUTIONS.

FOUNDATIONS

- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REVIEW THE GEOTECHNICAL REPORT PRIOR TO BIDDING FOR CONSTRUCTION PROCEDURES REQUIRED DUE TO EXISTING CONDITIONS SUCH AS PLASTIC SOILS, UNACCEPTABLE FILL, ETC.
- CONTINUOUS WALL FOOTINGS HAVE BEEN PROPORTIONED FOR A NEW ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. SPREAD FOOTING HAVE BEEN PROPORTIONED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF.
- SOIL BEARING PRESSURE IS BASED ON THE GEOTECHNICAL REPORT DATED FEB 26, 2025. FURNISHED BY SCI ENGINEERING, INC.
- GEOTECHNICAL ENGINEER SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF ALL FOUNDATION AND/OR SLAB BEARING STRATA.
- CONTRACTOR SHALL REMOVE AND REPLACE UNACCEPTABLE SOILS IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. ALL ORGANIC MATERIAL AND SOILS WHICH "PUMP" AFTER PROOF ROLLING WITH A FULLY LOADED TRUCK SHALL BE REMOVED.
- BOTTOM OF FOOTINGS MUST EXTEND 1'-6" BELOW PRESENT GRADE OR INTO "ENGINEERED FILL" AND 2'-6" BELOW PROPOSED GRADE UNLESS NOTED OTHERWISE IN GEOTECHNICAL REPORT.
- ENGINEERED FILL, ALL FILL MATERIAL SHALL BE SELECTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. EXISTING ON SITE MATERIALS SUCH AS THE NEAR-SURFACE FILL SOILS (SILTS AND CLAYS) SHOULD NOT BE USED AS ENGINEERED FILL MATERIALS.
- UNLESS NOTED OTHERWISE IN GEOTECHNICAL REPORT, EARTH FILL PLACEMENT SHOULD BE COMPACTED TO A DRY DENSITY OF NOT LESS THAN 95% OF THE STANDARD PROCTOR, AND WELL GRADED GRANULAR FILL SHOULD BE COMPACTED TO DRY DENSITY OF NOT LESS THAN 100% OF THE STANDARD PROCTOR. FILL SHALL BE PLACED IN LAYERS NOT EXCEEDING A LOOSE THICKNESS OF 8 INCHES.
- FOUNDATION WALL OR GRADE BEAMS HAVING EARTH PLACED ON EACH SIDE SHALL BE FILLED SIMULTANEOUSLY TO MAINTAIN A COMMON ELEVATION.
- CONCRETE FOOTINGS PLACED IN EARTH TRENCHED FORMS SHALL BE FREE OF STANDING WATER AND FROST. CONCRETE FOOTINGS SHALL BE PROTECTED FROM FREEZING FOR A PERIOD OF NOT LESS THAN 5 DAYS.

HEAVY GAUGE STEEL FRAMING

STANDARDS:

- 1.1. "AISI NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING" GENERAL PROVISIONS AISI S200-12.
- 1.2. "AISI NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING" PRODUCT DATA AISI S201-12.
- 1.3. "AISI NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING" WALL STUD DESIGN 2007 EDITION WITH SUPPLEMENT 1 AISI S211-07. (REAFFIRMED 2012)
- 1.4. "AISI NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING" HEADER DESIGN, AISI S212-07 (REAFFIRMED 2012)
- 1.5. "AISI NORTH AMERICAN STANDARDS FOR COLD-FORMED STEEL FRAMING" LATERAL DESIGN AISI S213-07 w/S1-09 (2012) 2007 EDITION WITH SUPPLEMENT (REAFFIRMED 2012)
- 1.6. "AISI NORTH AMERICAN STANDARDS FOR COLD-FORMED STEEL FRAMING" FLOOR AND ROOF SYSTEM DESIGN AISI S210-07 (2012) 2007 EDITION (REAFFIRMED 2012)
- 1.7. "AISI NORTH AMERICAN STANDARDS FOR COLD-FORMED STEEL FRAMING" TRUSS DESIGN AISI S214-12 2012 EDITION
- 1.8. SSMA - STEEL STUD MANUFACTURES ASSOCIATION "PRODUCT TECHNICAL GUIDE" LATEST EDITION

ALL FRAMING MEMBERS SHALL BE FORMED FROM CORROSION RESISTANT STEEL CORRESPONDING TO THE REQUIREMENTS:

2.1 NON-STRUCTURAL STUD FRAMING:	ASTM A653 / G60 COATING
2.2 STRUCTURAL STUD FRAMING:	ASTM A1003 TYPE 'H' / G60 COATING

FOLLOWING MINIMUM YIELD STRENGTHS:

12, 14, & 16 ga. JOISTS AND STUD	50 ksi
18 ga. AND LIGHTER	33 ksi

ALL INTERIOR AND EXTERIOR WALL FRAMING SHALL BE DELEGATED DESIGN

- 3.1 ENGAGE A QUALIFIED PROFESSIONAL ENGINEER TO DESIGN ALL EXTERIOR AND INTERIOR NON LOAD BEARING METAL STUD FRAMING.
- 3.2 DEFLECTION LIMITS:
 - 3.2.1 EXTERIOR STUDS: L/600 SUPPORTING VENEER; L/360 ALL OTHERS
 - 3.2.2 HEADERS: TOTAL LOAD: L/360; LIVE LOAD L/600 WITH MAXIMUM 1/4"
- 3.3 PROVIDE DETAILS OF ALL HEADERS AND CONNECTIONS TO SUPPORTING STRUCTURE SHOWN ON DOCUMENTS.
- 3.4 DEFLECTION TRACKS AND BYPASS CLIPS SHALL BE DESIGNED TO ALLOW FOR VERTICAL DEFLECTIONS OF L/360 OF THE SUPPORTING MEMBER

SCREWED CONNECTIONS

- 4.1. USE A SELF-DRILLING #10 TEK SCREW WHERE INDICATED ON THE DRAWINGS.
- 4.2. A MINIMUM OF THREE (3) EXPOSED THREADS SHALL PENETRATE THROUGH ALL JOINTED MATERIAL.
- 4.3. SCREWS USED TO ATTACH EXTERIOR MATERIALS SHALL BE CADMIUM-PLATED CORROSION RESISTANT FASTENERS.

WELDED CONNECTIONS

- 5.1. ALL CONNECTIONS SHALL BE WELDED U.N.O. WELDING SHALL BE IN ACCORDANCE WITH (AWS)D1.3, STRUCTURAL WELDING CODE.
- 5.2. SHIELDED METAL ARC WELDING (SMAW) SHALL BE USED FOR 18 ga. AND HEAVIER MEMBERS. AWS E-6012, E-6013 OR E-7014 ELECTRODES OF 3/32" OR 1/8" DIAMETER SHALL BE USED. WELDING EQUIPMENT HEAT SETTING SHALL BE VARIED DEPENDENT ON MATERIAL THICKNESS.

CUTTING STEEL FRAMING MEMBERS SHALL BE DONE WITH A SAW OR CUTTING SHEARS. TORCH CUTTING OF LOAD BEARING MEMBERS IS NOT PERMITTED.

ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR AN ANGULAR FIT TIGHT AGAINST ABUTTING MEMBERS.

ALL FRAMING SHALL BE GALVANIZED. TOUCH UP ALL WELDS AND DAMAGED AREAS WITH APPROVED ZINC RICH PAINT GALVANIZING TOUCH UP PAINT.

WHERE STUDS FRAME TO STRUCTURAL ROOF OR FLOOR MEMBERS SUBJECT TO DEFLECTION FROM OCCUPANT LOADING, EITHER A DEEP LEG DOUBLE TRACK, 'Z'-CLIP, OR DEEP SLOTTED LEG TRACK ASSEMBLY SHALL BE PROVIDED AT THE TOP RUNNER TO ACCOMMODATE VERTICAL STRUCTURAL MOVEMENT.

STUDS SHALL BE PLUMBED, ALIGNED, AND SECURELY ATTACHED TO BOTH TOP AND BOTTOM RUNNERS. SPLICES IN STUDS ARE NOT PERMITTED. ALIGNMENT OF STUDS (PLUMBNESS) AND WALLS (STRAIGHTNESS) SHALL BE WITHIN 1/960th OF RESPECTIVE HEIGHTS AND LENGTHS.

PROVIDE 1 ROW OF BRIDGING @ 6'-0" AT WALL STUDS UP TO 10'-0" TALL AND AT A MAXIMUM SPACING OF 4'-0" ABOVE 10'-0" TALL.

AT METAL STUD BEARING WALLS PROVIDE 10" OF UNPUNCHED STEEL AT BOTH ENDS OF STUD.

STUDS SHALL BE STOCKPILED AT THE JOB SITE IN A VERTICAL POSITION RESTING ON THEIR FLANGES, AND SHALL BE ADEQUATELY SUPPORTED ON WOOD BLOCKING TO KEEP STUDS FREE FROM MUD AND DIRT.

POST INSTALLED ANCHORS

DEFINITIONS:

WEDGE ANCHOR: THREADED STUD ANCHOR WITH AN EXPANSION CONE AND EXPANDING WEDGE TYPE CLIPS.

UNDERCUT ANCHOR: THREADED STUD TYPE ANCHOR THAT PERFORM SELF-UNDERCUTTING. UNDERCUT PORTION OF ANCHOR MUST HAVE A PROJECTED BEARING AREA 2.5 TIMES THE BOLT DIAMETER.

ADHESIVE ANCHOR: TWO PART ACRYLIC EPOXY ADHESIVE WITH MIXING NOZZLE. THREADED ANCHOR ROD SHALL MEET ASTM A36. SCREEN TUBE MUST BE USED FOR HOLLOW CMU APPLICATIONS.

SCREW ANCHOR: ONE PIECE ANCHOR WITH FIXED HEAD AND THE ANCHOR BODY HAS A SCREW TYPE THREADED DESIGN.

POST INSTALLED ANCHORS SHALL BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. MANUFACTURER'S WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS, FOR PRODUCTS TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES (MINIMUM) AS REQUIRED BY THE BUILDING CODE.

1. INSTALLATION OF ANCHORS SHALL FOLLOW THE LATEST INFORMATION REGARDING TORQUE AND INSTALLATION SPECIFICATIONS FROM THE MANUFACTURE OF THE PRODUCTS.
2. POST INSTALLED ANCHORS SHALL BE INSTALLED ONLY WHERE SPECIFIED ON THE STRUCTURAL DOCUMENTS.
3. INSTALLATION OF POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS SHALL BE APPROVED BY THE ENGINEER OF RECORD.
4. REINFORCING BARS IN THE CONCRETE STRUCTURE SHALL NOT BE CUT IN ORDER TO INSTALL POST-INSTALLED ANCHORS, UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
5. SUBMITTAL OF ALL PROPOSED PRODUCTS, WITH THE TECHNICAL DATA AND CURRENT ICC-ESR REPORTS IS REQUIRED FOR REVIEW AND APPROVAL BY ENGINEER OF RECORD.
6. ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED Installation INSTRUCTIONS IN CONJUNCTION WITH EDGE DISTANCE, SPACING AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS.
7. CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION TRAINING FOR ALL PRODUCTS TO BE USED, PRIOR TO COMMENCEMENT OF WORK. ONLY TRAINED INSTALLERS SHALL PERFORM POST-INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND BE MADE AVAILABLE TO THE ARCHITECT/ENGINEER OF RECORD AS REQUESTED.
8. ADHESIVE ANCHORS INSTALLED HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI 318-14 17.8.2.2) PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER OF RECORD FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
9. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI 318-14 17.8)
10. ANCHORAGE APPLICATIONS:
 - 10.1 CONCRETE:
 - 10.2 GROUTED SOLID CONCRETE MASONRY:
 - 10.3 HOLLOW CONCRETE MASONRY:
 - 10.4 MULTI-WYTHE BRICK MASONRY:
11. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE BUILDING CODE AND PER THE CURRENT ICC-ES REPORT (IBC 2015/2018 TABLE 1705.3 NOTE 4)
12. ANCHOR TESTING:
 - 12.1 MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED, UN-CRACKED AND SEISMIC CONCRETE RECOGNITION.
 - 12.2 ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ASTM E 488/ACI 355.1 AND ICC-ES AC308 FOR CRACKED, UN-CRACKED AND SEISMIC CONCRETE RECOGNITION.

DR. R. F. RUGG (C) 2022 RKC TECH ENGINEERING, INC.
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AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR
ACCURACY, COMPLETENESS AND STATUS.

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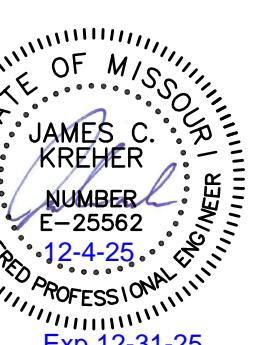
Revisions

GENERAL NOTES

S0_11

ISSUE DATE 12-24-2021

JOB NUMBER: 25-021



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Revisions:

SPECIAL INSPECTIONS

S0.12

ISSUE DATE: 12.04.2025

JOB NUMBER: 25-021

SPECIAL STRUCTURAL INSPECTIONS

1. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A CERTIFIED INSPECTOR APPROVED BY THE ARCHITECT/ENGINEER OF RECORD AND THE BUILDING OFFICIAL. THE SPECIAL INSPECTOR OR AGENCY SHOULD BE UNDER THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER SPECIALIZING IN STRUCTURAL ENGINEERING.
2. THE CONTRACTOR IS RESPONSIBLE FOR SCHEDULING AND TIMELY NOTIFICATION OF THE NEED FOR SPECIAL INSPECTION.
3. DUTIES OF THE SPECIAL INSPECTOR:
 - 3.1. THE SPECIAL INSPECTOR WILL OBSERVE THE ASSIGNED ITEMS FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
 - 3.2. THE SPECIAL INSPECTOR WILL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND THE ENGINEER/ARCHITECT OF RECORD WITHIN 48 HOURS AFTER COMPLETING INSPECTIONS.
 - 3.3. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE ENGINEER/ARCHITECT.
 - 3.4. UPON COMPLETION OF THE WORK, THE SPECIAL INSPECTOR SHALL COMPLETE AND SIGN A FINAL REPORT CERTIFYING THAT TO THE BEST OF THE INSPECTORS KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS, SPECIFICATIONS AND PROVISION OF THE IBC CODE.
4. INSPECTIONS: REFER TO THE IBC BUILDING CODE FOR THE DEFINITION OF PERIODIC AND CONTINUOUS INSPECTIONS INCLUDING SPECIFIC REQUIREMENTS.
5. ALL SPECIAL INSPECTIONS PERFORMED ON THIS PROJECT SHALL COMPLY WITH 2018 IBC SECTIONS 1704 AND 1705
 - 5.1. SPECIAL INSPECTION DAILY LOGS/REPORTS SHALL BE MAINTAINED ON-SITE BY THE PROJECT SUPERINTENDENT FOR USE AND REFERENCE BY THE EUREKA, MO INSPECTION STAFF.
 - 5.2. SUPERINTENDENT SHALL FORWARD ALL INSPECTION REPORTS TO ARCHITECT AND ENGINEER OF RECORD PRIOR TO COMPLETING "CERTIFICATE OF SPECIAL INSPECTION" FOR SUBMISSION TO THE EUREKA, MO INSPECTION STAFF FOR THE FINAL BUILDING INSPECTION.

STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS - SEISMIC RESISTANCE - STRUCTURAL

VERIFICATION AND INSPECTION	EXTENT: CONTINUOUS PERIODIC SUBMITTAL	REFERENCE STANDARD	IBC REFERENCE	AGENT QUALIFICATION
IBC SECTION 1705.12.1 THROUGH 1705.12.9, UNLESS EXEMPTED BY THE EXCEPTIONS OF SECTION 1704.2.				
1. STRUCTURAL STEEL: SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE SHALL BE IN ACCORDANCE WITH SECTION 1705.12.1.1 OR 1705.12.1.2, AS APPLICABLE.				
1.1. SEISMIC FORCE-RESISTING SYSTEMS: SPECIAL INSPECTIONS OF STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY B, C, D, E, OR F, SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.	P	AISC 341	IBC 1705.12.1.1	PE/SE OR EIT

STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS - WIND RESISTANCE - STRUCTURAL

VERIFICATION AND INSPECTION	EXTENT: CONTINUOUS PERIODIC SUBMITTAL	REFERENCE STANDARD	IBC REFERENCE	AGENT QUALIFICATION
IBC SECTION 1705.11.1 THROUGH 1705.11.3, UNLESS EXEMPTED BY THE EXCEPTIONS OF SECTION 1704.2.				
1. WIND-RESISTING COMPONENTS: PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS:				
1.1. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.	P		IBC 1705.11.3	PE/SE OR EIT

STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS - CONCRETE CONSTRUCTION

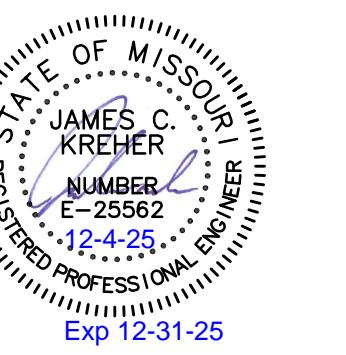
VERIFICATION AND INSPECTION	EXTENT: CONTINUOUS PERIODIC SUBMITTAL	REFERENCE STANDARD	IBC REFERENCE	AGENT QUALIFICATION
IBC SECTION 1705.3				
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	P	ACI 318: Ch20, 25.2 25.3, 26.5.1-26.5.3	IBC 1908.4	PE/SE OR EIT
2. REINFORCING BAR WELDING				
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706	P			
b. INSPECT SINGLE-PASS FILLET WELD, MAXIMUM 5/16"	P	ACI 318: 26.5.4 AWS D1.4	IBC 1705.3.1	AWS-CW1
c. INSPECT ALL OTHER WELDS.	C			
3. INSPECT ANCHORS CAST IN CONCRETE.	P	ACI 318: 17.8.2	IBC 1901.3	PE/SE OR EIT
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS				
4.1. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	P	ACI 318: 17.8.2.4		ACI-STT
4.2. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN SECTION 4.1.	P	ACI 318: 17.8.2		
5. VERIFY USE OF REQUIRED DESIGN MIX	P	ACI 318: Ch19 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3	ACI-CFTT OR ACI-CCI
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	C	ASTM C 172 ASTM C 31 ACI 318: 26.4.5, 26.12	IBC 1908.10	ACI-CFTT OR ACI-STT
7. INSPECT CONCRETE PLACEMENT FOR PROPER APPLICATIONS TECHNIQUES	C	ACI 318: 26.4.5	IBC 1908.6, 1908.7, 1908.8	ACI-CFTT OR ACI-CCI
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURES AND TECHNIQUES.	C		IBC 1908.9	ACI-CFTT OR ACI-LTT
9. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	P	ACI 318: 26.10.1(b)		

**STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS
FABRICATION AND IMPLEMENTATION PROCEDURES - STRUCTURAL STEEL**

VERIFICATION AND INSPECTION	EXTENT: CONTINUOUS PERIODIC SUBMITTAL	REFERENCE STANDARD	IBC REFERENCE	AGENT QUALIFICATION
IBC SECTION 1705.2				
1. FABRICATION PROCEDURES: REVIEW OF FABRICATORS WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. AT COMPLETION OF FABRICATION SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.	S	FABRICATOR SHALL SUBMIT ONE OF THE TWO QUALIFICATIONS	IBC 1705	
- OR -				
2. AISC CERTIFICATION				
3. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING CODE OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.	S		IBC 1705	

STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS - SOILS AND FOUNDATION CONSTRUCTION

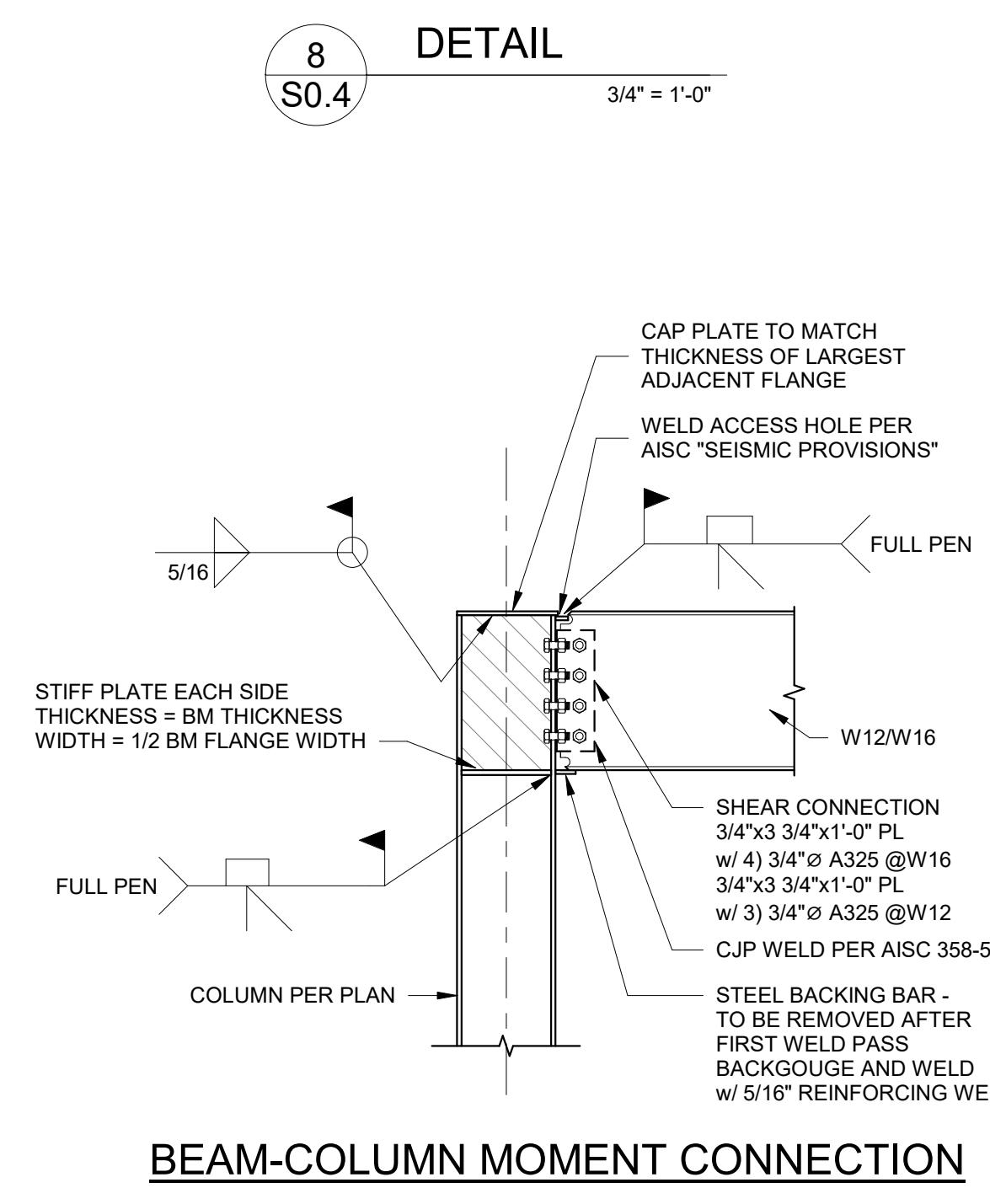
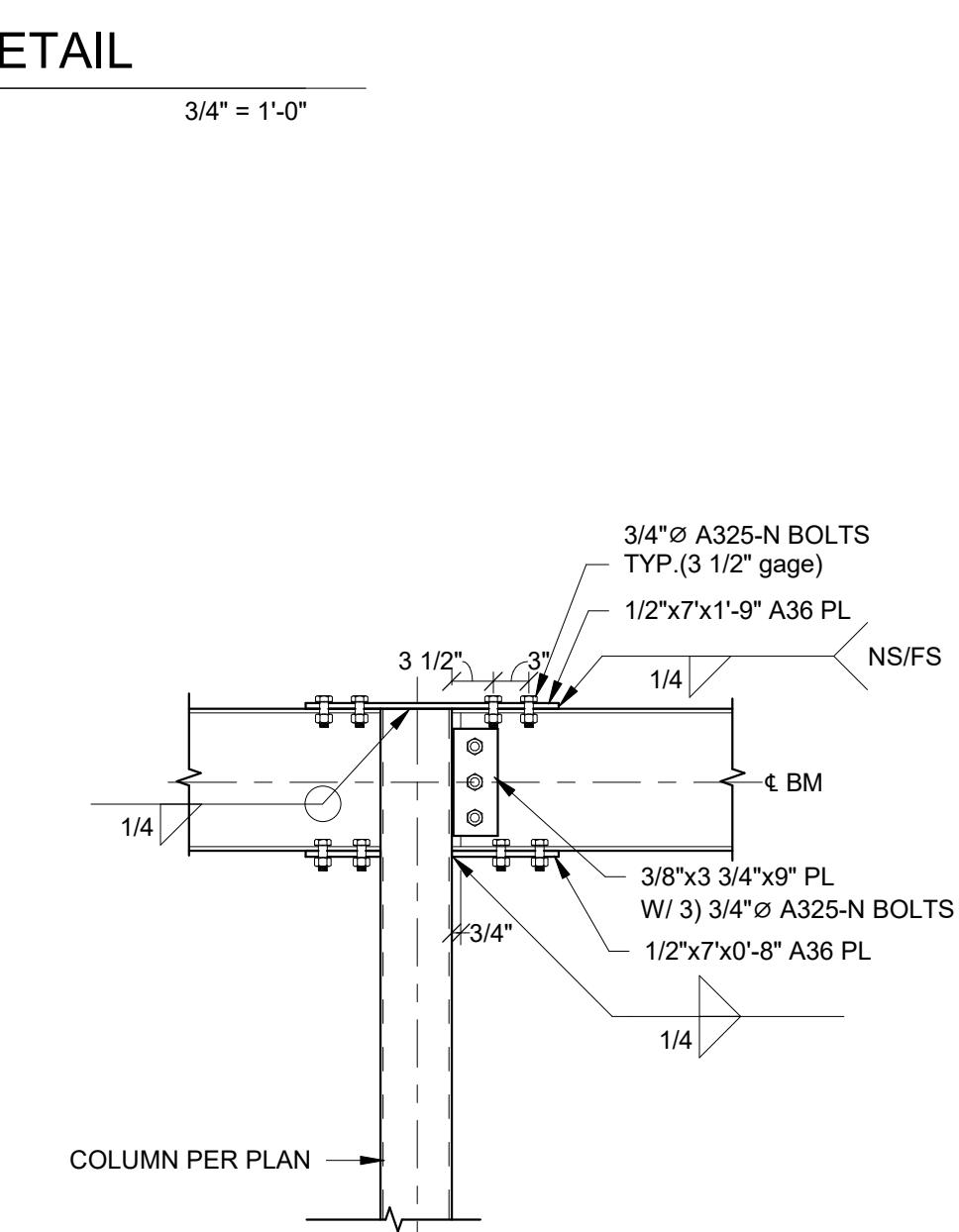
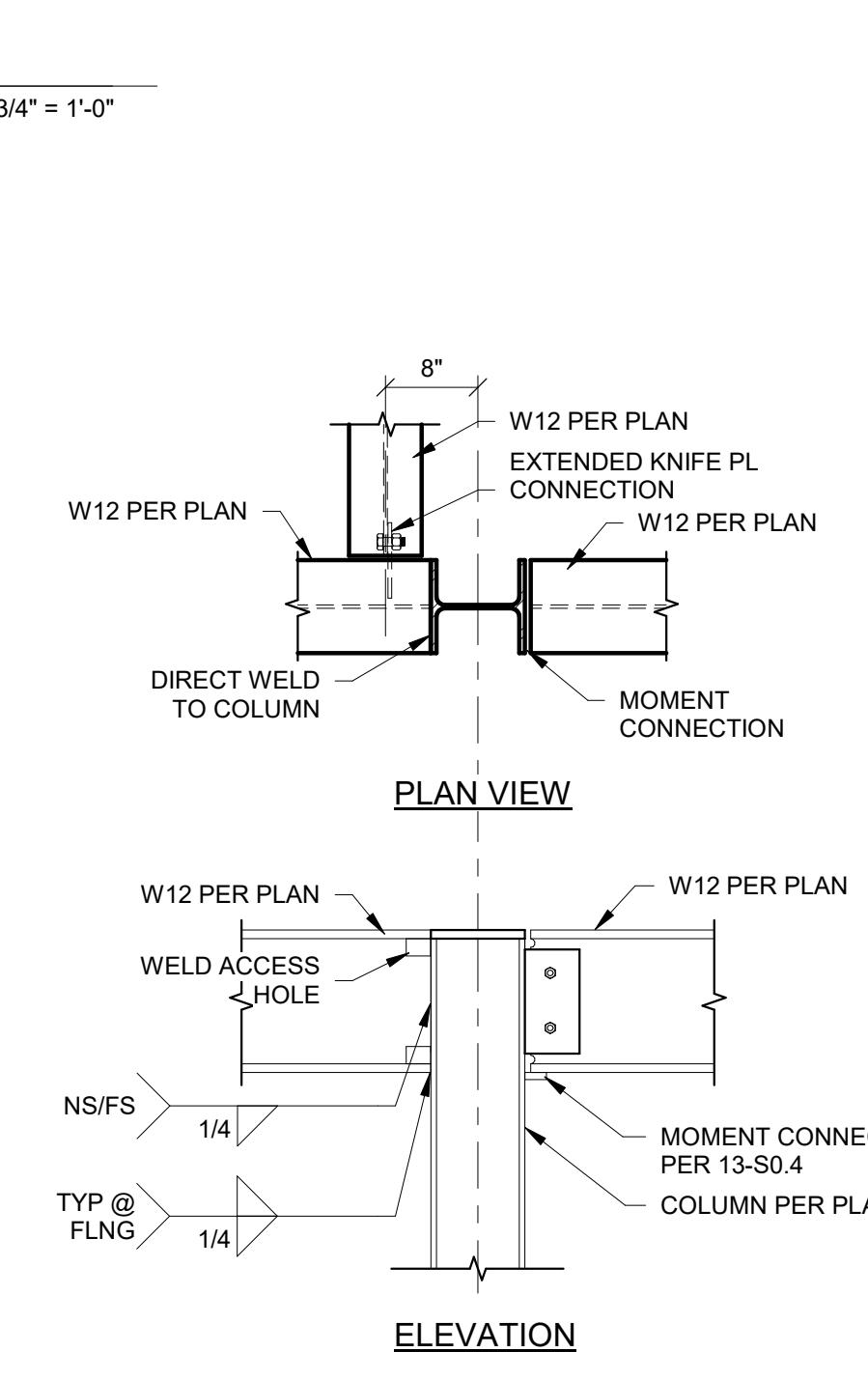
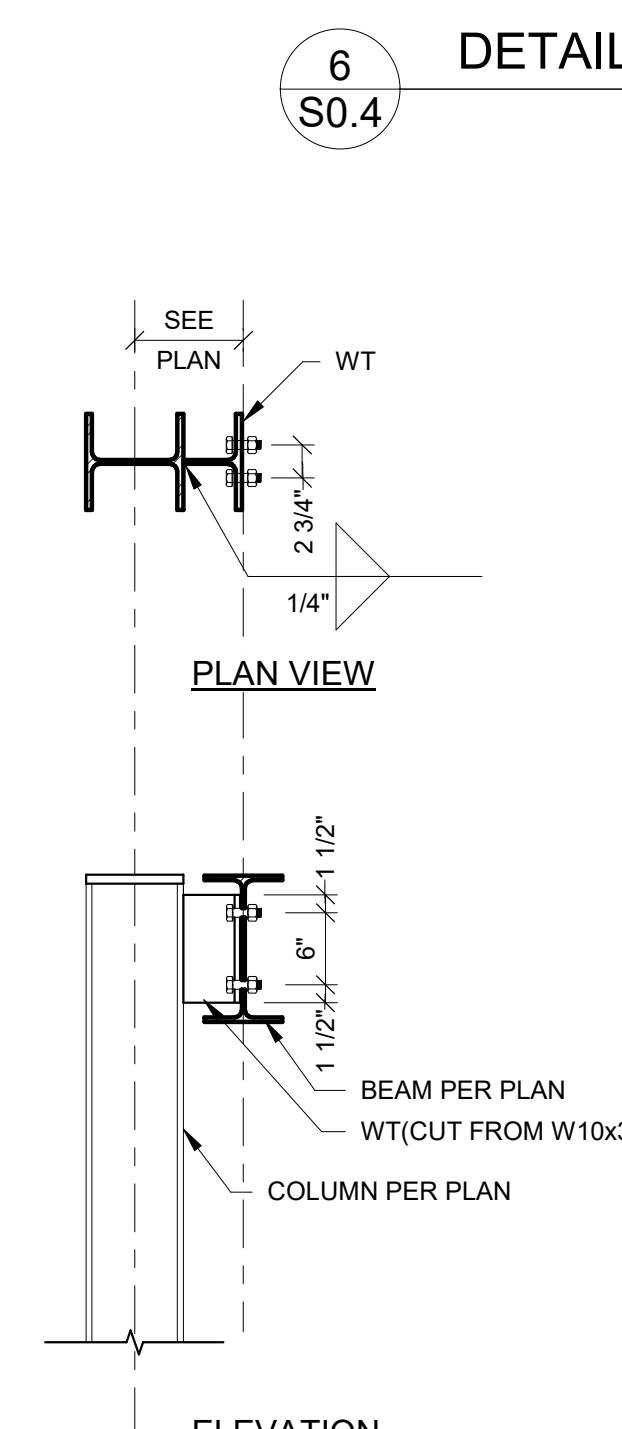
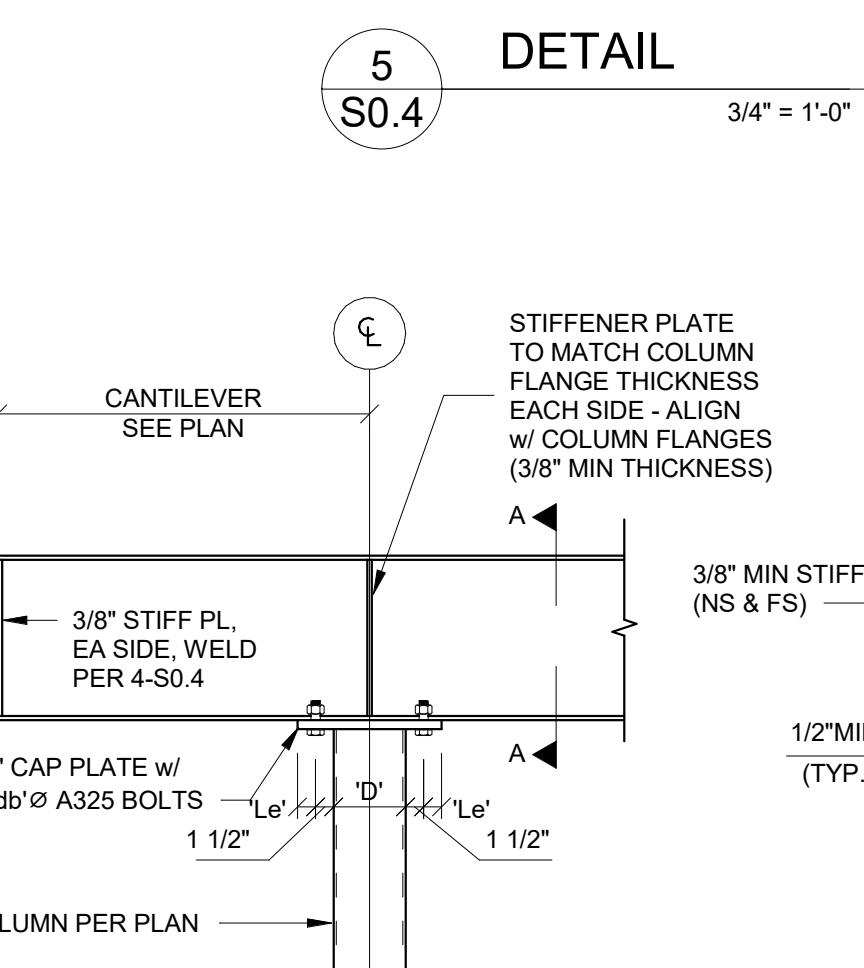
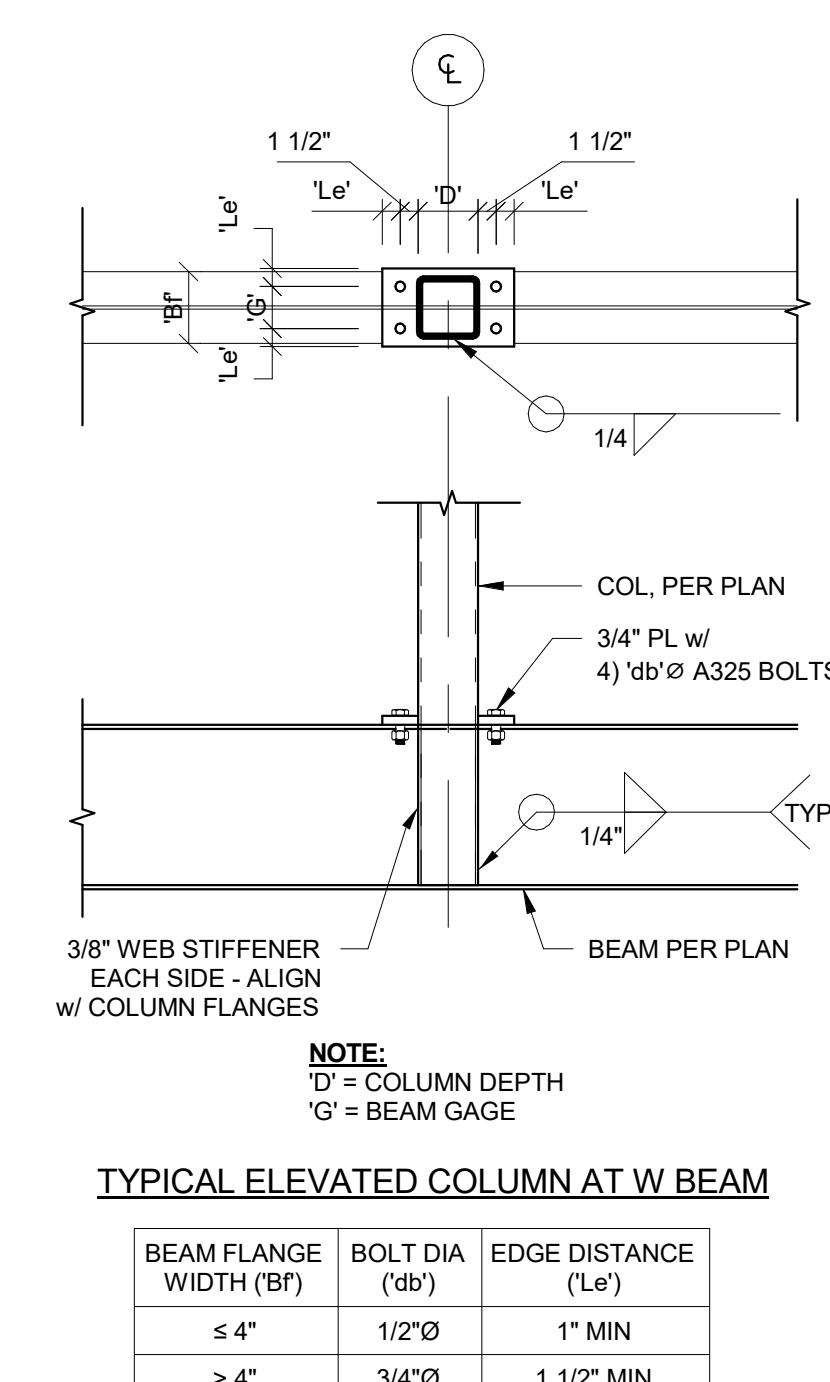
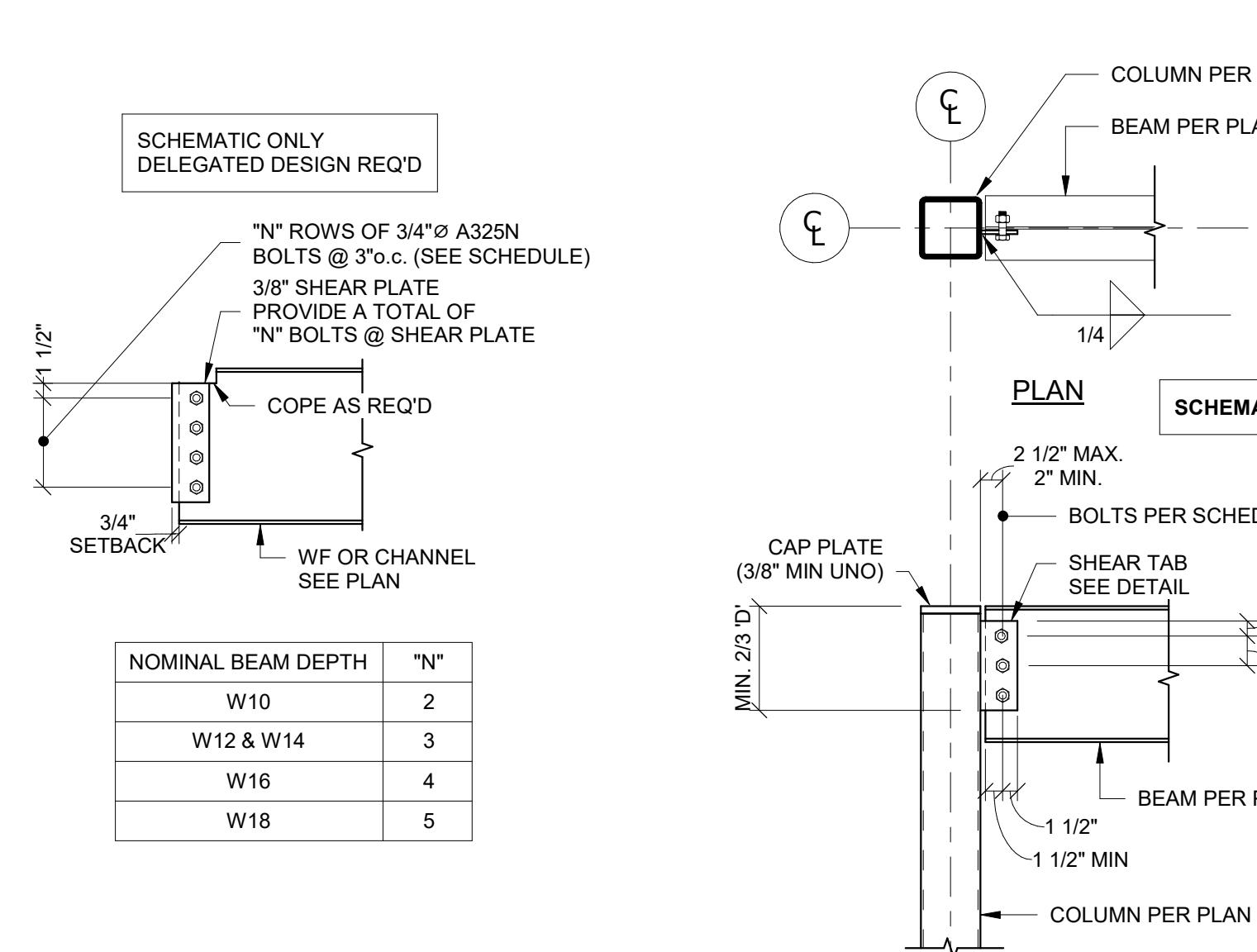
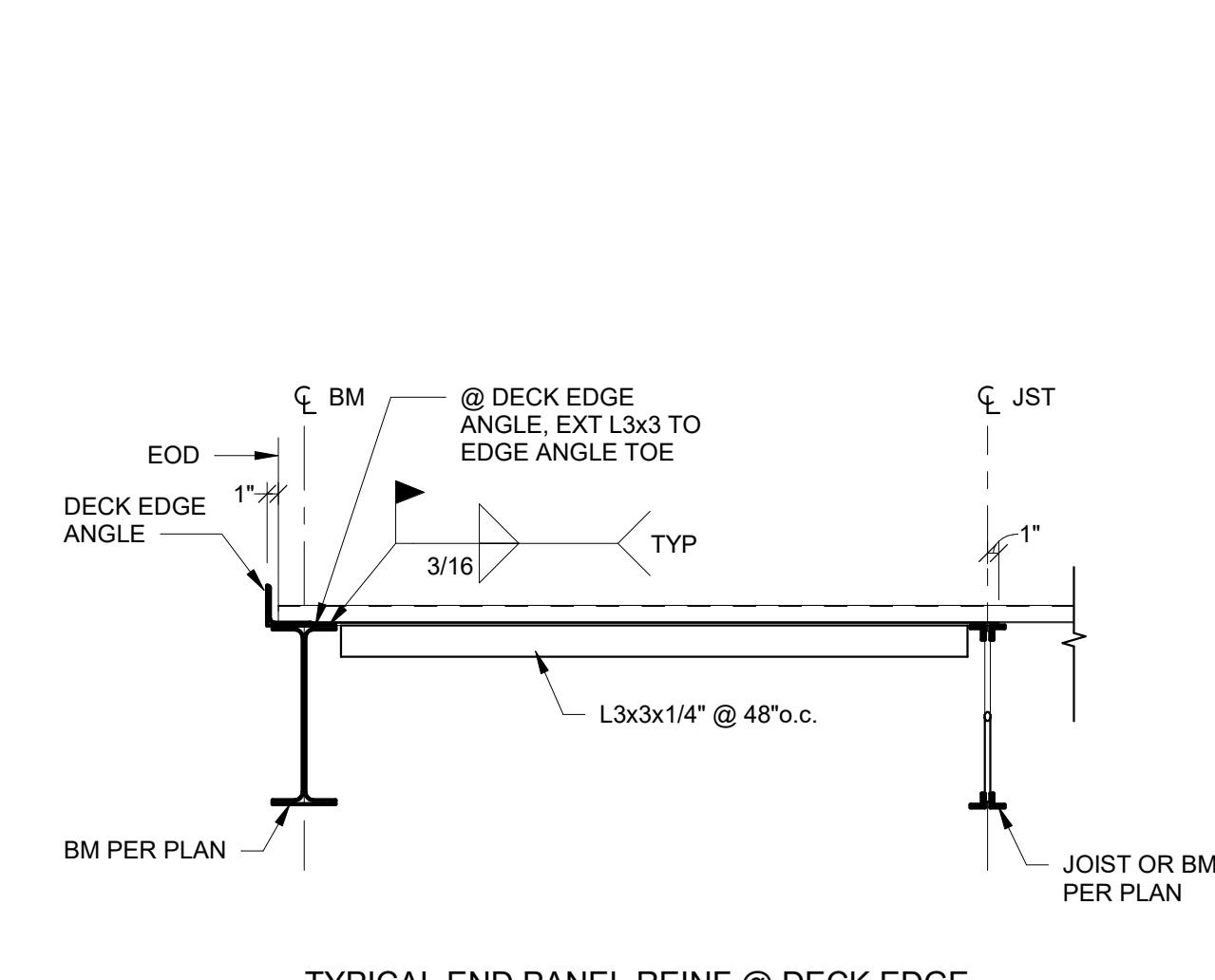
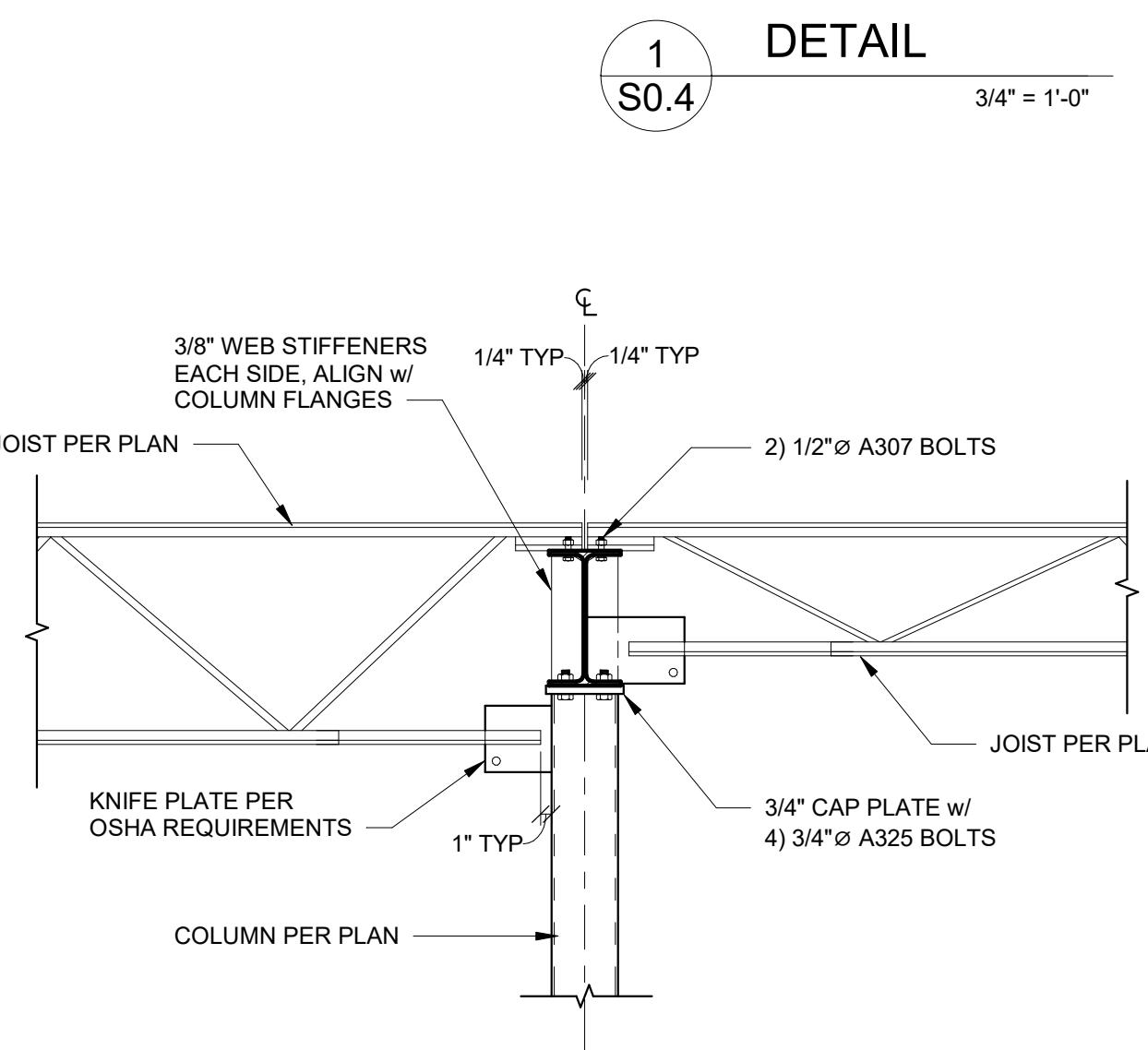
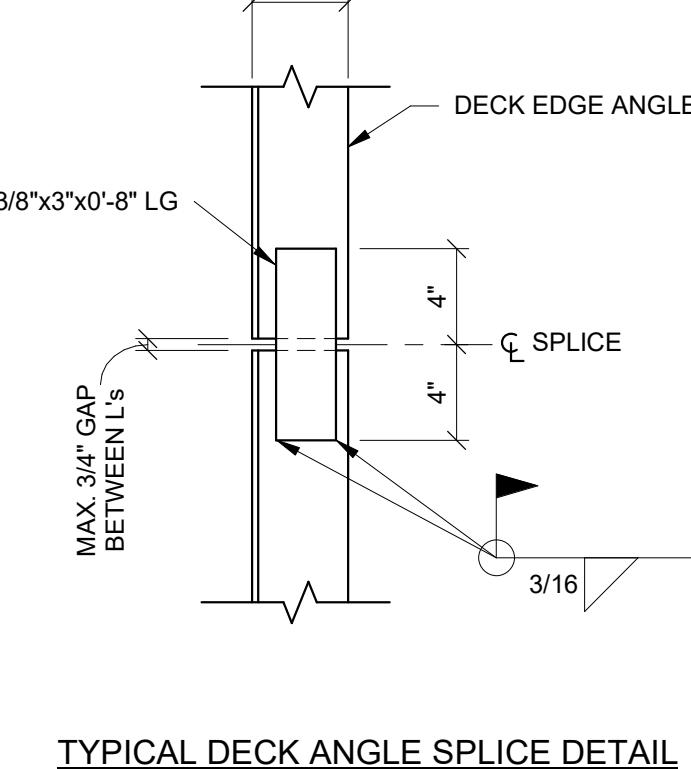
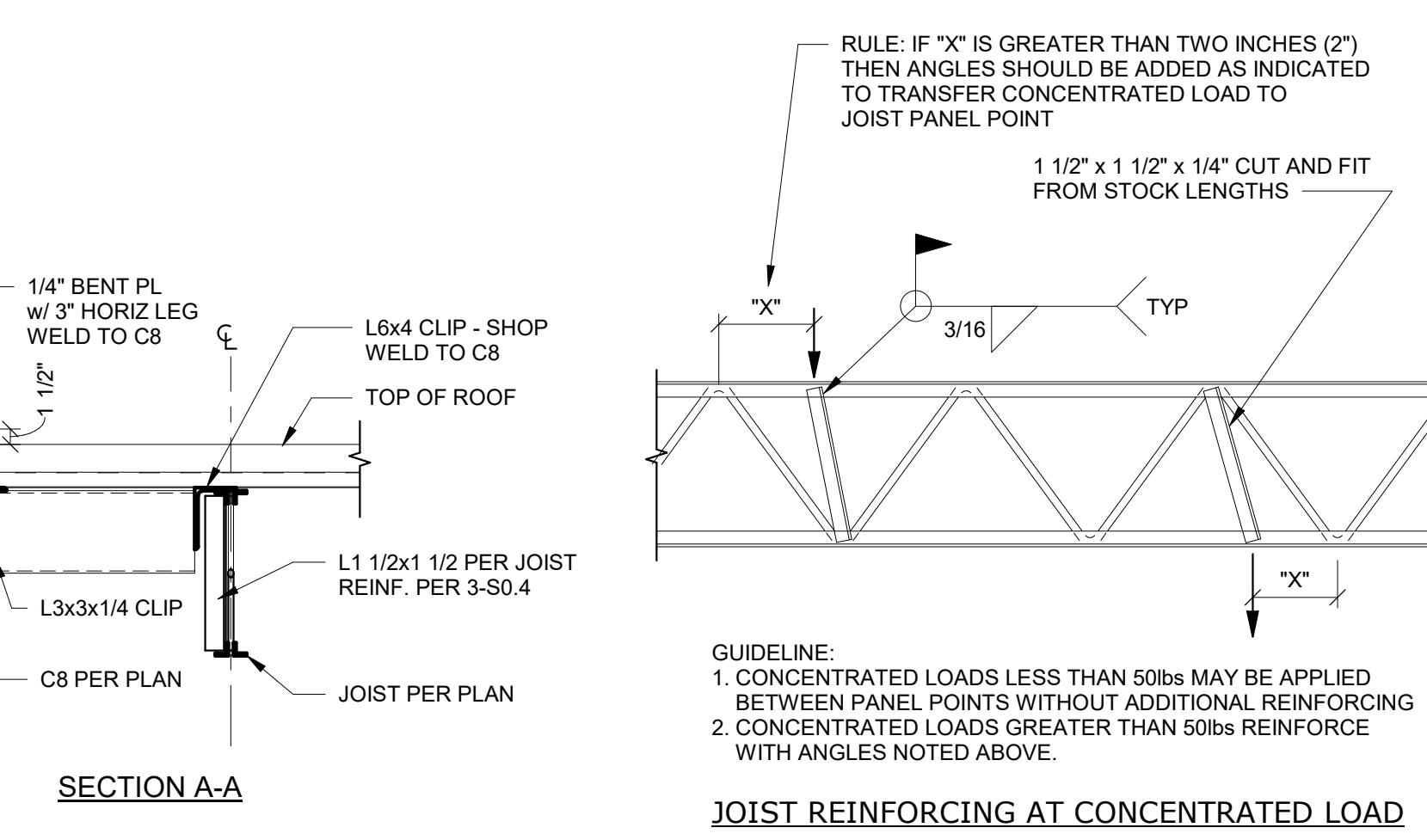
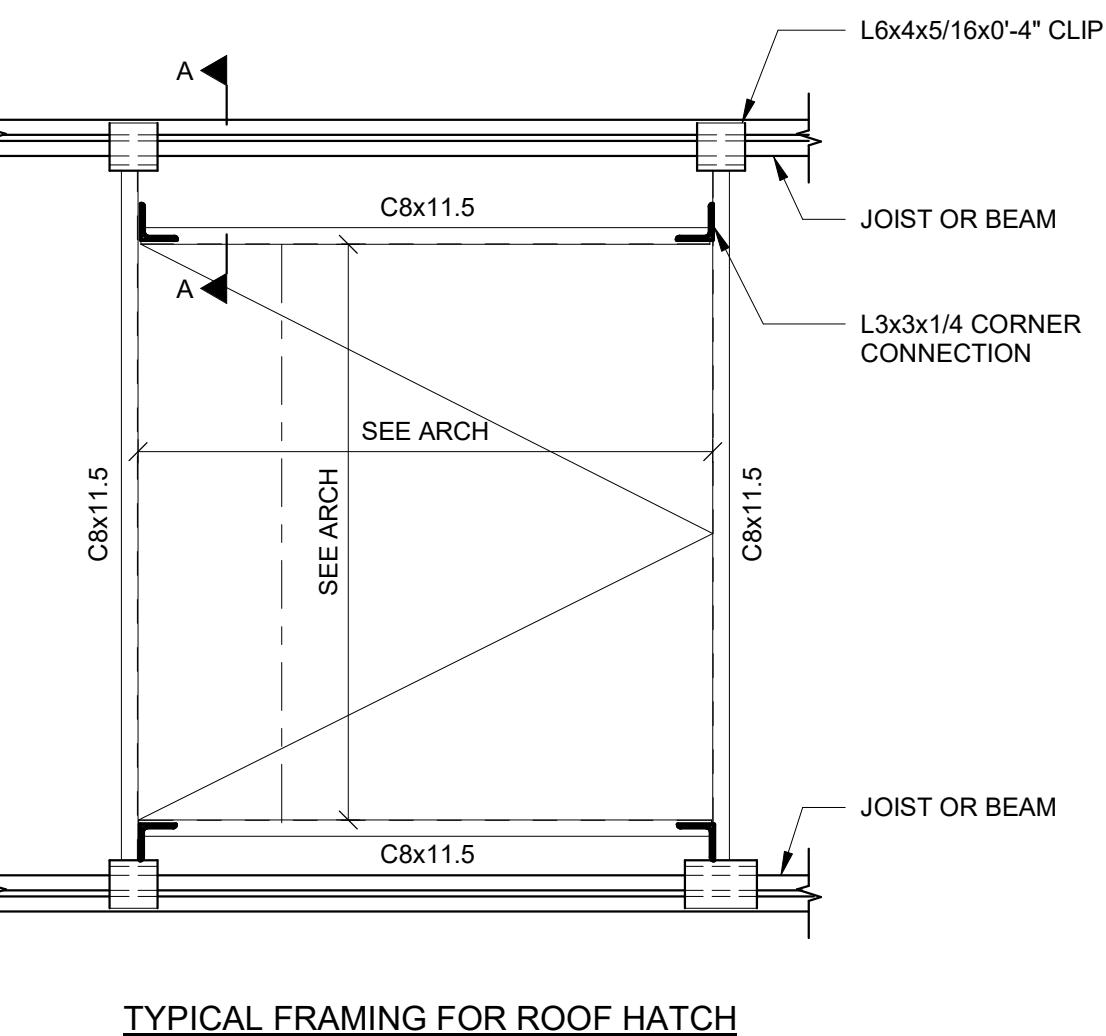
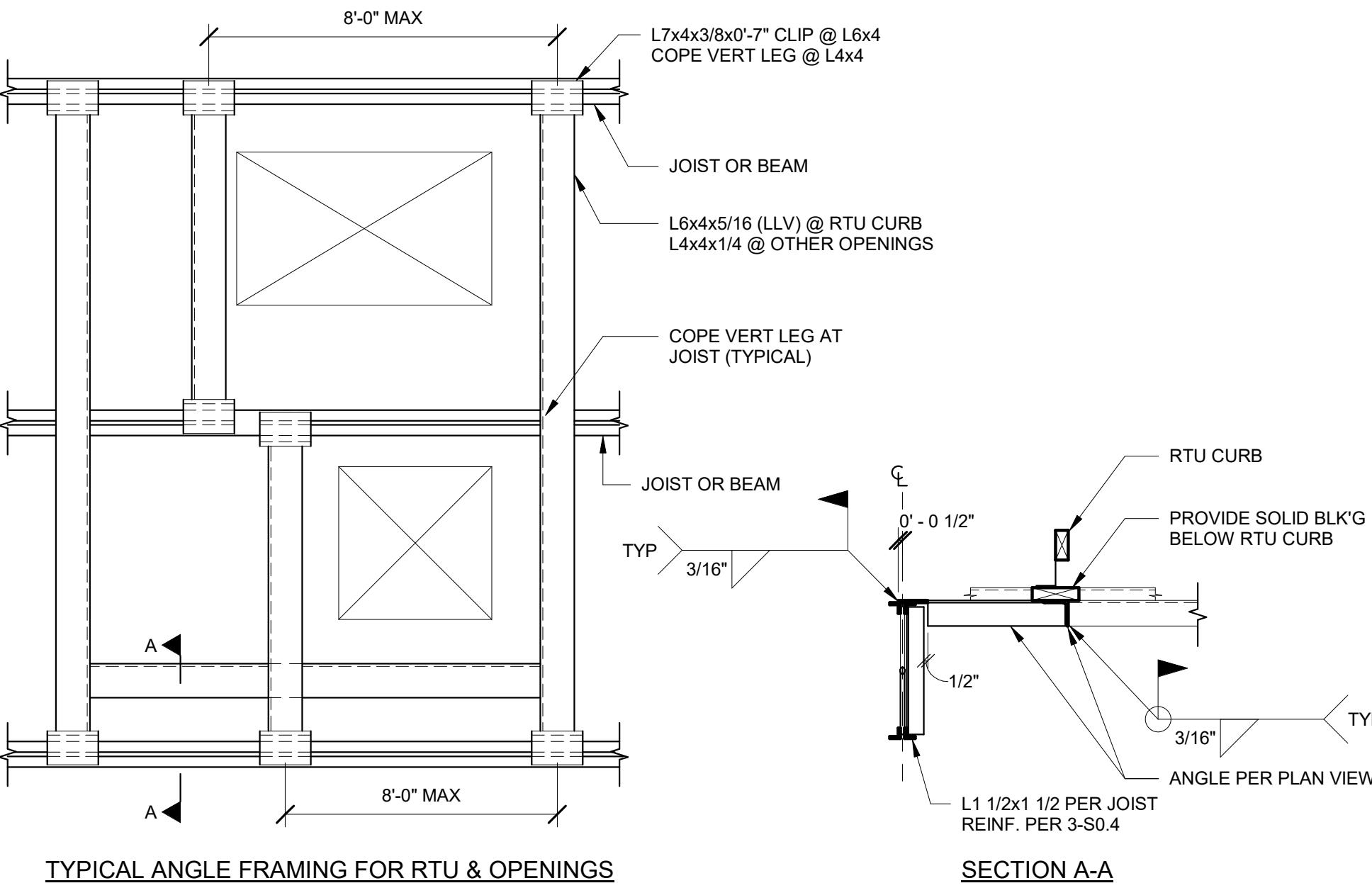
VERIFICATION AND INSPECTION	EXTENT: CONTINUOUS PERIODIC SUBMITTAL	REFERENCE STANDARD	IBC REFERENCE	AGENT QUALIFICATION
IBC SECTION 1705.6; 1705.7; 1705.8; 1705.9 IBC TABLE 1705.6; 1705.7; 1705.8				
1. VERIFY EXISTING SOIL CONDITIONS, FILL PLACEMENT AND LOAD BEARING REQUIREMENTS.				
1.1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	P		IBC 1705.6	PE/GE; EI OR ET
1.2. VERIFY EXCAVATION ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	P		IBC 1705.6	PE/GE; EI OR ET
1.3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	P		IBC 1705.6	PE/GE; EI OR ET
1.4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	C		IBC 1705.6	PE/GE; EI OR ET
1.5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT THE SITE HAS BEEN PREPARED PROPERLY.	P		IBC 1705.6	PE/GE; EI OR ET



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Revisions:

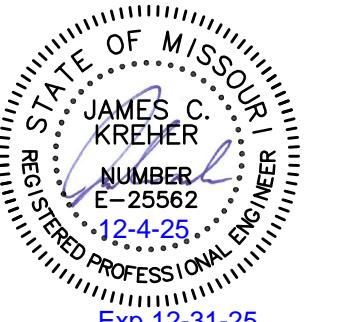
FRAMING TYPICAL DETAILS S0.4



BEAM FLANGE WIDTH (Bf)	BOLT DIA (db)	EDGE DISTANCE (Le)
≤ 4"	1/2"	1" MIN
> 4"	3/4"	1 1/4" MIN

DETAIL

3/4" = 1'-0"



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Revisions:



TRASH ENCLOSURE PLAN

$$1/4" = 1'-0"$$

1. SLAB CONSTRUCTION: 6" CONCRETE SLAB ON GRADE REINFORCED w/ #4s @ 16" o.c.E.W. AND 4" COMPACTED GRANULAR FILL (< 12% FINES).
2. ELEVATIONS ARE REFERENCED FROM CIVIL PLANS.
SEE ARCHITECTURAL OR SITE DRAWINGS FOR ACTUAL SITE ELEVATIONS.
3. CONTINUOUS FOOTINGS HAVE BEEN PROPORTIONED FOR A NET ALLOWABLE BEARING PRESSURE OF 2000 PSF. ISOLATED FOOTING HAVE BEEN PROPORTIONED FOR A NET ALLOWABLE BEARING PRESSURE OF 2500 PSF. BEARING PRESSURE SHALL BE VERIFIED BY A GEOTECHNICAL ENGINEER BEFORE FOOTINGS ARE PLACED. CONTRACTOR SHALL REMOVE AND REPLACE UNACCEPTABLE SOILS IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. ALL SOILS WHICH "PUMP" SHALL BE REMOVED.
4. FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATION AND CONDITIONS. NOTIFY ARCHITECT / ENGINEER IF ACTUAL EXISTING CONDITIONS CONFLICT WITH THE INFORMATION SHOWN OR IMPLIED ON THE DRAWINGS

Diagram illustrating the dimensions and reference points for a Stem Wall Footing (FTG):

- IDTH**: Vertical dimension from the base to the top of the stem wall.
- DEPTH**: Vertical dimension from the base to the bottom of the stem wall.
- XXX'-XX" FTG**: The main label for the footing.
- TF=XXX'-XX"**: The total height of the stem wall.
- TOP OF FOOTING REFERENCED FROM CIVIL**: The reference point for the top of the stem wall.

Site plan diagram showing a proposed grade line. The diagram includes a north arrow pointing up, a total width of TW=504.25', and a total length of BL=498.92'. The proposed grade line is indicated by a diagonal line with a slope of 5% (S1.01). The diagram also shows a vertical distance of 8" and a thickness of 4" for a wall section. The total length is also marked as BL=496.25' at the bottom right. Foundation dimensions of 2' - 4" are indicated at the bottom.

Elevation 1 - a

1/4" = 1'-0"

Elevation 2 - a

1/4" = 1'-0"

Site plan diagram showing a proposed building footprint and grade levels. The diagram includes the following data points:

- Top Left: TW=504.25'
- Top Right: BL=501.58'
- Bottom Left: BL=496.25'
- Bottom Left: TF=494.92'
- Bottom Center: 20' - 0"

The diagram shows a proposed building footprint with a stepped foundation. The proposed grade is indicated by a diagonal line with a slope indicator. The building footprint is bounded by vertical lines on the left and right, and a horizontal line at the bottom.

Elevation 3 - a

1/4" = 1'-0"

Architectural elevation drawing showing a building section. The total width is TW=504.2. The section is divided into three parts: two 5' - 0" wide wings on the sides and a central 10' - 0" wide entrance. Foundation levels are BL=496.25 and TF=494.92.

Elevation 4 - a

1/4" - 1" O"

Architectural section drawing of a brick veneer wall. The wall is 7" thick and 8' 0" high. The top 5" is cap stone coping. The wall has dovetail slots at 24" o.c. vertically with anchors at 16" o.c. The brick veneer is applied per arch. The foundation has #4 continuous bars, #5 diamond wire lacing at 12" o.c., and #4 diamond wire lacing at 24" o.c. The total foundation weight is TF=494.92'. The foundation has 3" clearances (CLR) on the sides and 2" (min) at the top. The slab is 6" thick with 1/2" compressible filler and a 2' - 6" lap. The elevation is BL=SEE ELEVATIONS and EL=SEE PLAN. The drawing shows a cross-section with dimensions 2' - 0", 1' - 0", 1' - 4", 4' - 4", and 5 1/2". A scale bar at the bottom indicates 3/4" = 1'-0".

5" 7"

CAP STONE COPING
SEE ARCH

DOVETAIL SLOTS
@ 24" o.c. VERT
w/ ANCHORS @ 16" o.c.

BRICK VENEER
PER ARCH

8' - 0"

5 1/2"

#4s HORIZ @ 12" o.c.

#5s VERT @ 12" o.c.

1/2" COMPRESSIBLE FILLER

6" SLAB PER CIVIL

EL=SEE PLAN

BL=SEE ELEVATIONS

#4 CONT

#5 DWLS @ 12" o.c.

#4 DWLS @ 24" o.c.

TF=494.92'

2"

2" CLR

1' - 4"

1' - 2"

3" CLR

3"

2' - 0" 1' - 0" 1' - 4"

4' - 4"

3) #5s CONT

#5s @ 16" o.c.

3) #5s CONT

SECTION

5

S1.01

3/4" = 1'-0"

SECTION

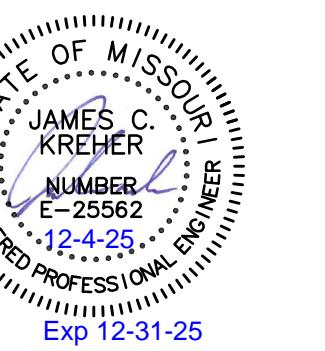
3/4" ≡ 1"

This architectural elevation drawing shows a foundation and a fence structure. The foundation is a rectangular concrete slab with dimensions of 20' - 0" by 10' - 0". The fence is made of horizontal rails and vertical posts. The fence posts are labeled 'WF52-14 TYP' and 'TF=494.92'. The fence height is indicated as 496.25'. The fence is supported by a concrete footing. The drawing includes several elevation labels: 'Elevation 1 - a' at the bottom left, 'Elevation 2 - a' on the left, 'Elevation 3 - a' at the top, and 'Elevation 4 - a' on the right. The labels 'S1.01' are placed in circles at the top, bottom, and right edges of the drawing. The drawing also includes a dimension of 5' - 0" for the width of the fence posts and a dimension of 4' - 0" for the distance between the fence and the foundation.

TRASH ENCLOSURE PLAN S1.01

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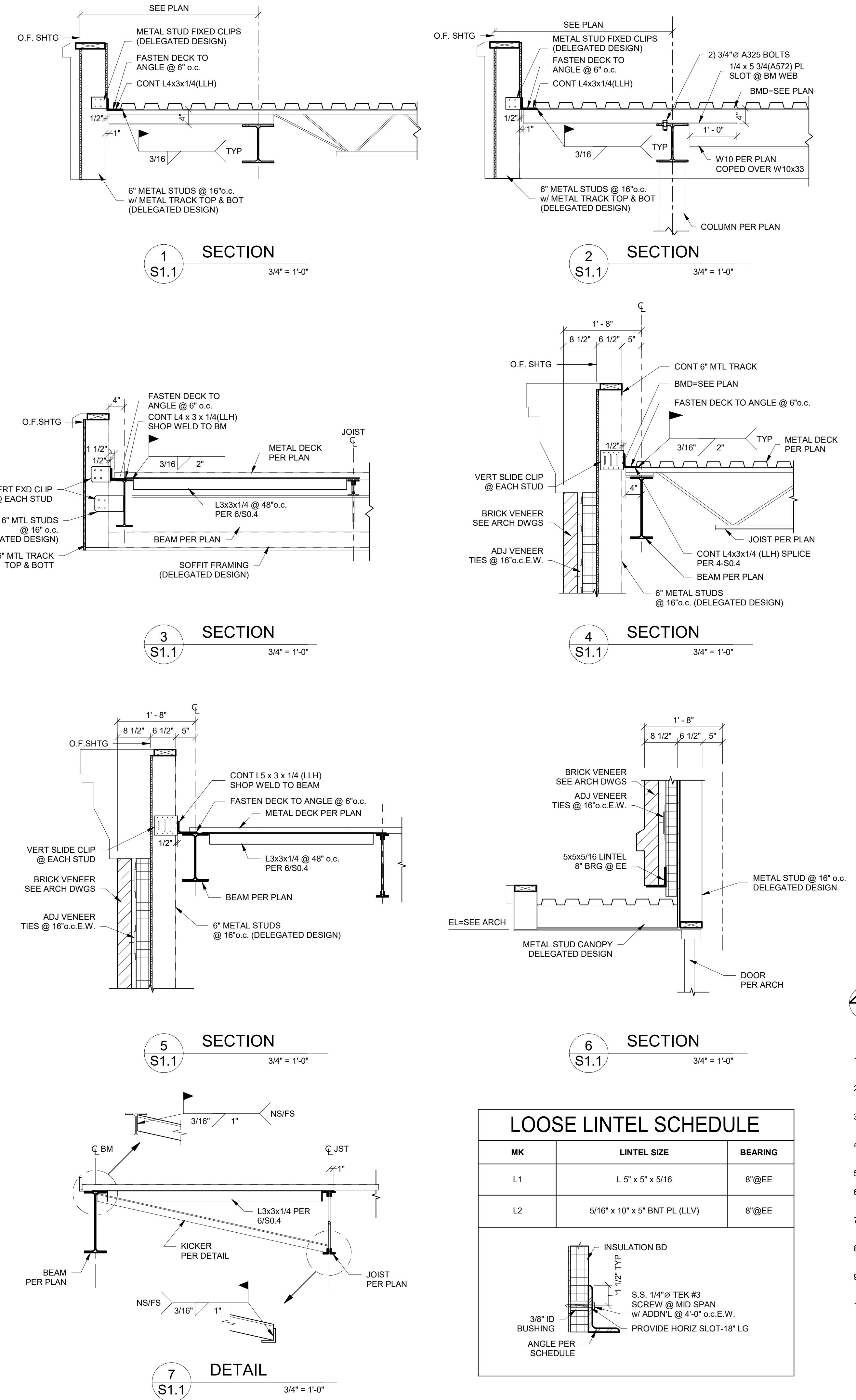


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Revisions:

LOW ROOF FRAMING PLAN

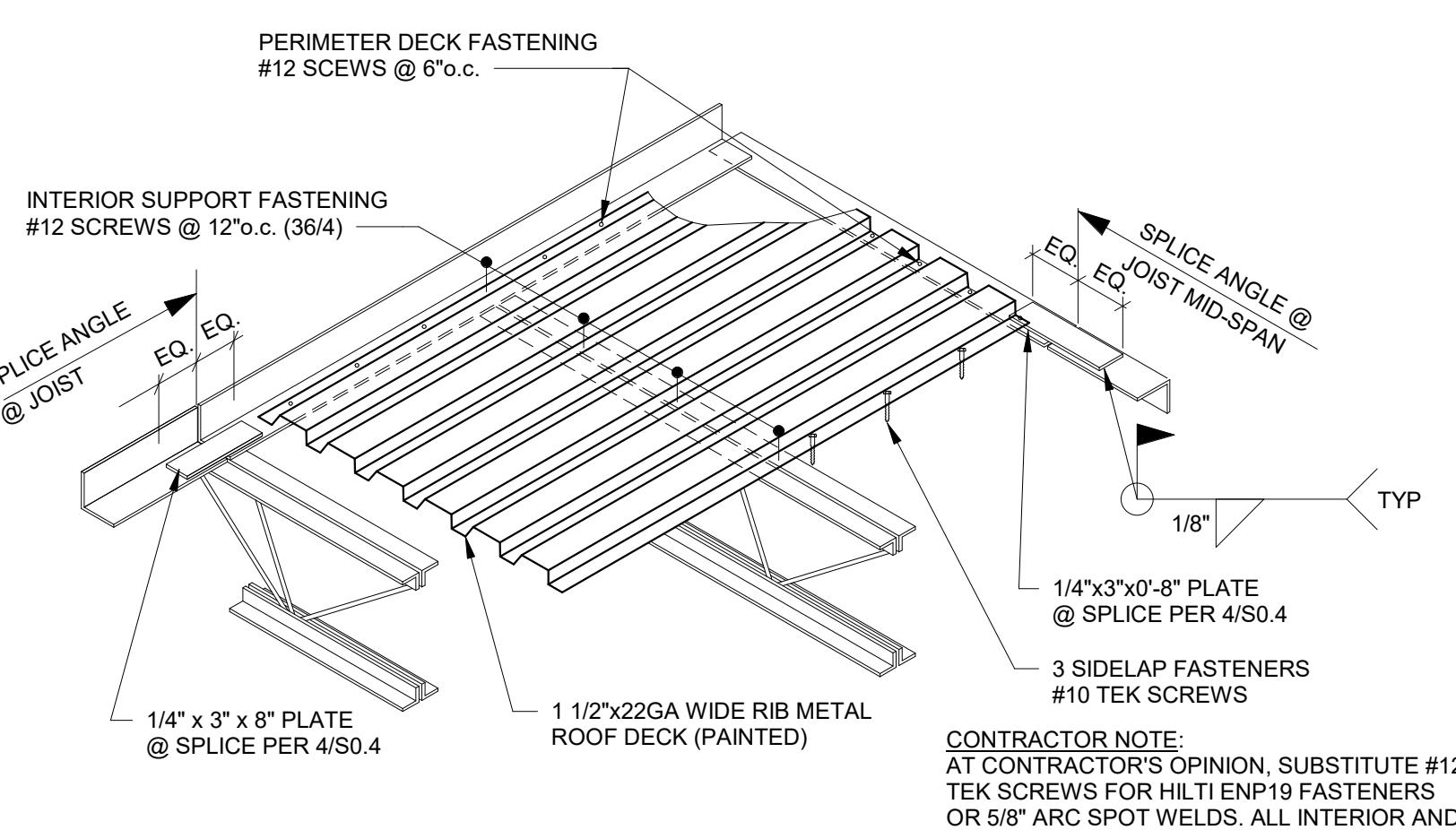
S1.1



LOW ROOF FRAMING PLAN

1/8" = 1'-0"

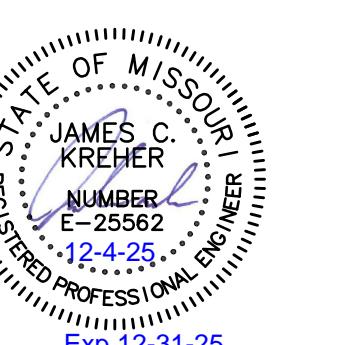
1. ROOF CONSTRUCTION: 1 1/2" DEEP BY 22 GA WIDE RIB PAINTED METAL DECK INSTALLED AND FABRICATED IN ACCORDANCE WITH SDI SPECIFICATIONS, UNLESS NOTED OTHERWISE. ATTACH DECK PER SECTION 8/S1.1.
2. BOTTOM OF METAL DECK ELEVATIONS IS REFERENCED FROM FINISH FLOOR ELEVATION EL = 100'-0" AND NOTED THUS (BMD = XXX-XX).
3. COORDINATE ROOF OPENINGS AND ANGLE FRAMING LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. OPENING LARGER THAN 12' x 12' AND RTU CURBS REQUIRE ANGLE FRAMES PER I/S0.4.
4. PROVIDE JOIST REINFORCING ANGLE FOR MISCELLANEOUS JOIST LOADS APPLIED TO TOP AND BOTTOM CHORDS PER DETAIL 3/S0.4.
5. K-SERIES JOIST SEAT DEPTH 2 1/2" UNLESS NOTED (X/X") ON PLAN
6. JOIST SUPPLIER SHALL DESIGN AND PROVIDE HORIZONTAL JOIST BRIDGING AT TOP AND BOTTOM CHORDS AS PER SJI RECOMMENDATION AND OSHA REQUIREMENTS.
7. STRUT END JOIST AT EACH COLUMN PER OSHA REQUIREMENTS, WHERE JOIST OCCURS AT COLUMNS PER SECTION 5/S0.4.
8. ► DENOTES MOMENT CONNECTION. SEE DETAIL 13/S0.4.
9. FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS AND CONDITIONS. NOTIFY ARCHITECT / ENGINEER IF ACTUAL EXISTING CONDITIONS CONFLICT WITH THE INFORMATION SHOWN OR IMPLIED ON THE DRAWINGS.
10. DESIGN ROOF LOAD: 42.5 PSF (DEAD LOAD = 20 PSF + LIVE/SNOW LOAD = 20 PSF + WIND LOAD=10 PSF). JOIST NEW UPLIFT: 20 PSF (UNFACTORDED).



CONTRACTOR NOTE:
AT CONTRACTOR'S OPTION, SUBSTITUTE #12 TEK SCREWS FOR HILTI ENP19 FASTENERS OR 5/8" ARC SPOT WELDS. ALL INTERIOR AND EXTERIOR FASTENERS SHALL BE THE SAME. A COMBINATION OF HILTI FASTENERS AND TEK SCREWS OR WELDS IS NOT ALLOWED.

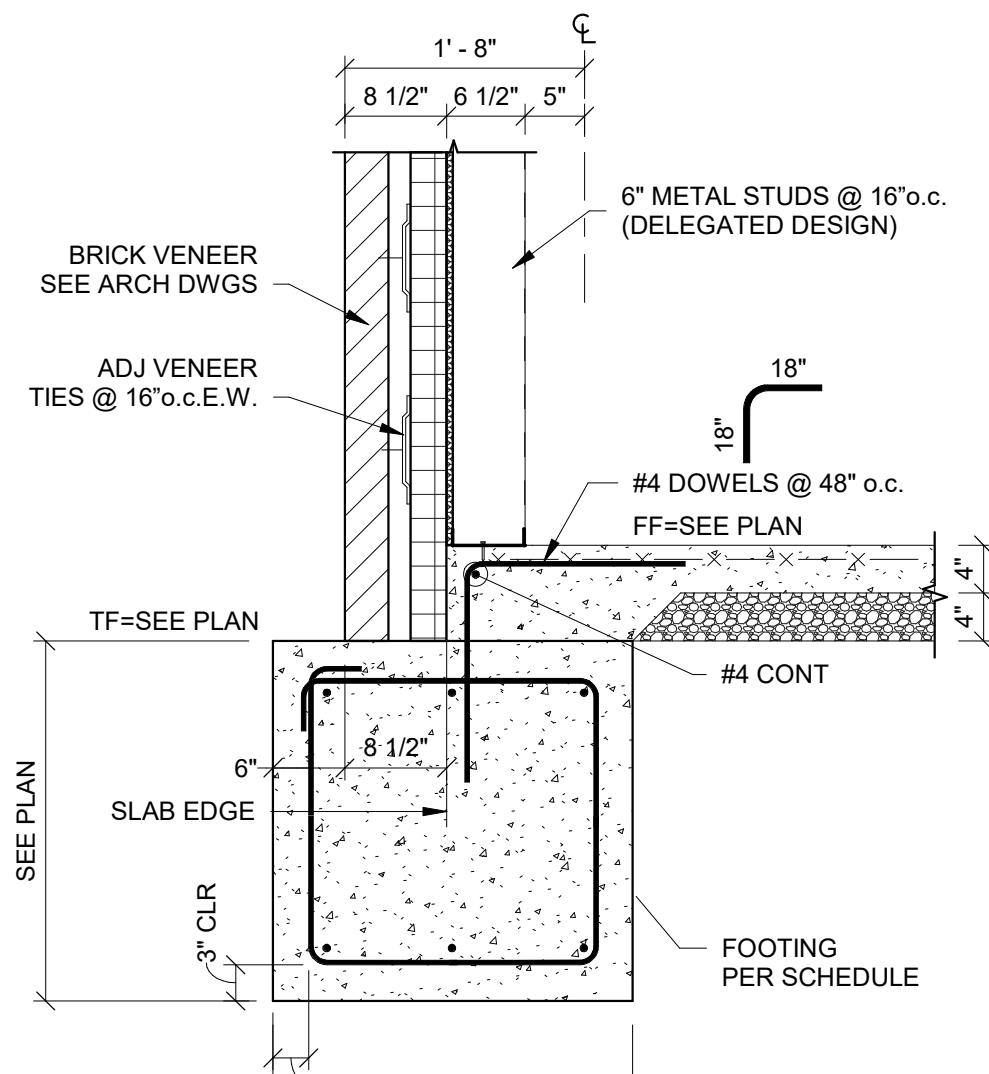
ISSUE DATE: 12.04.2025

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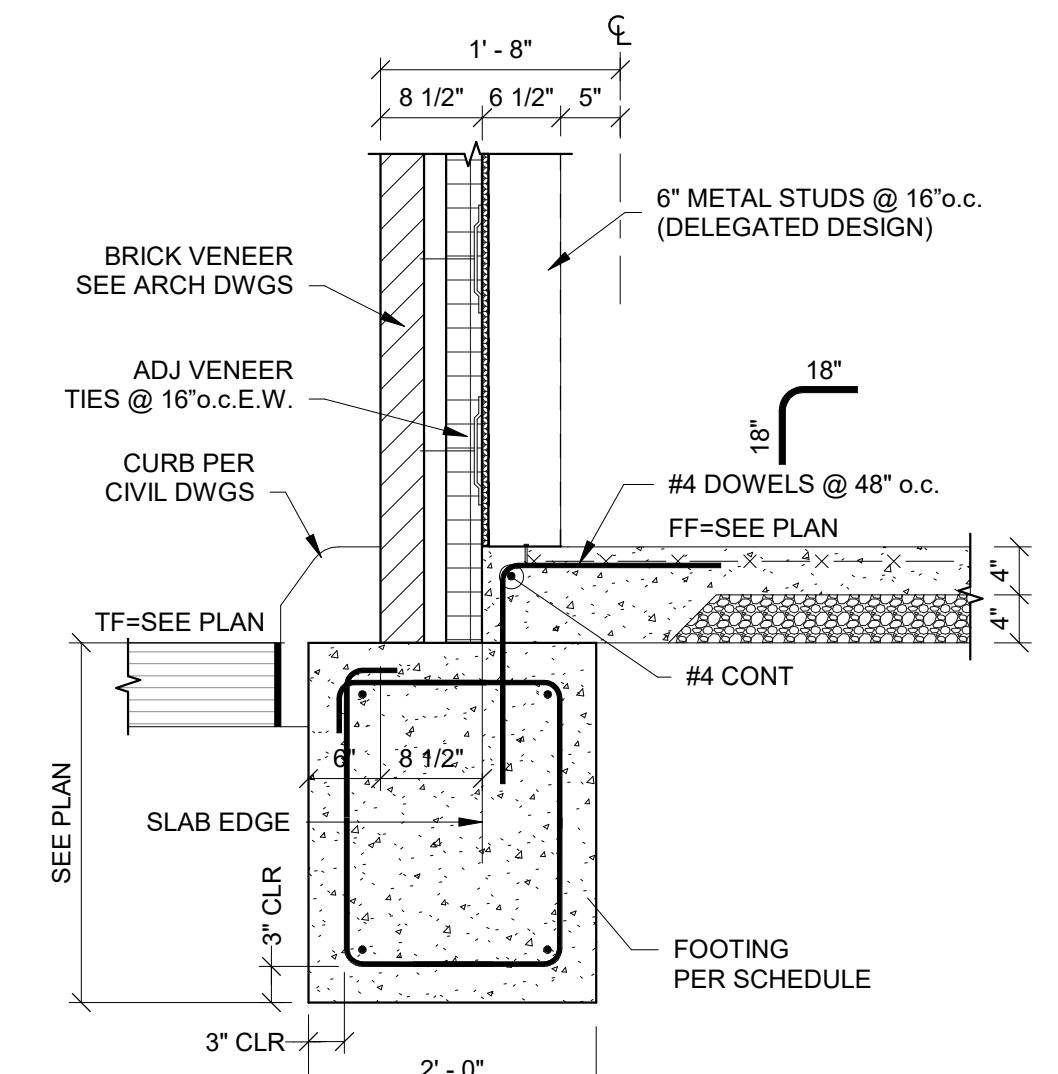


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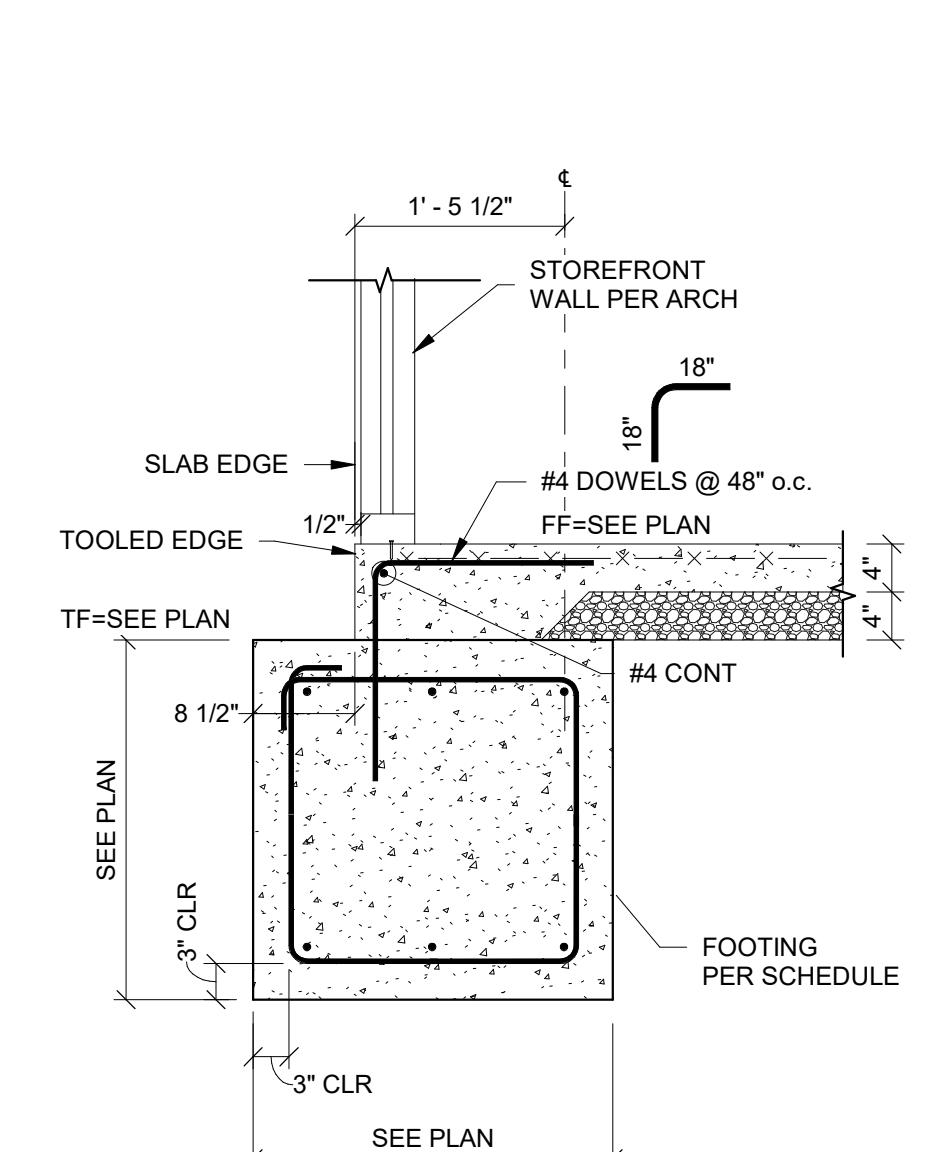
Revisions:



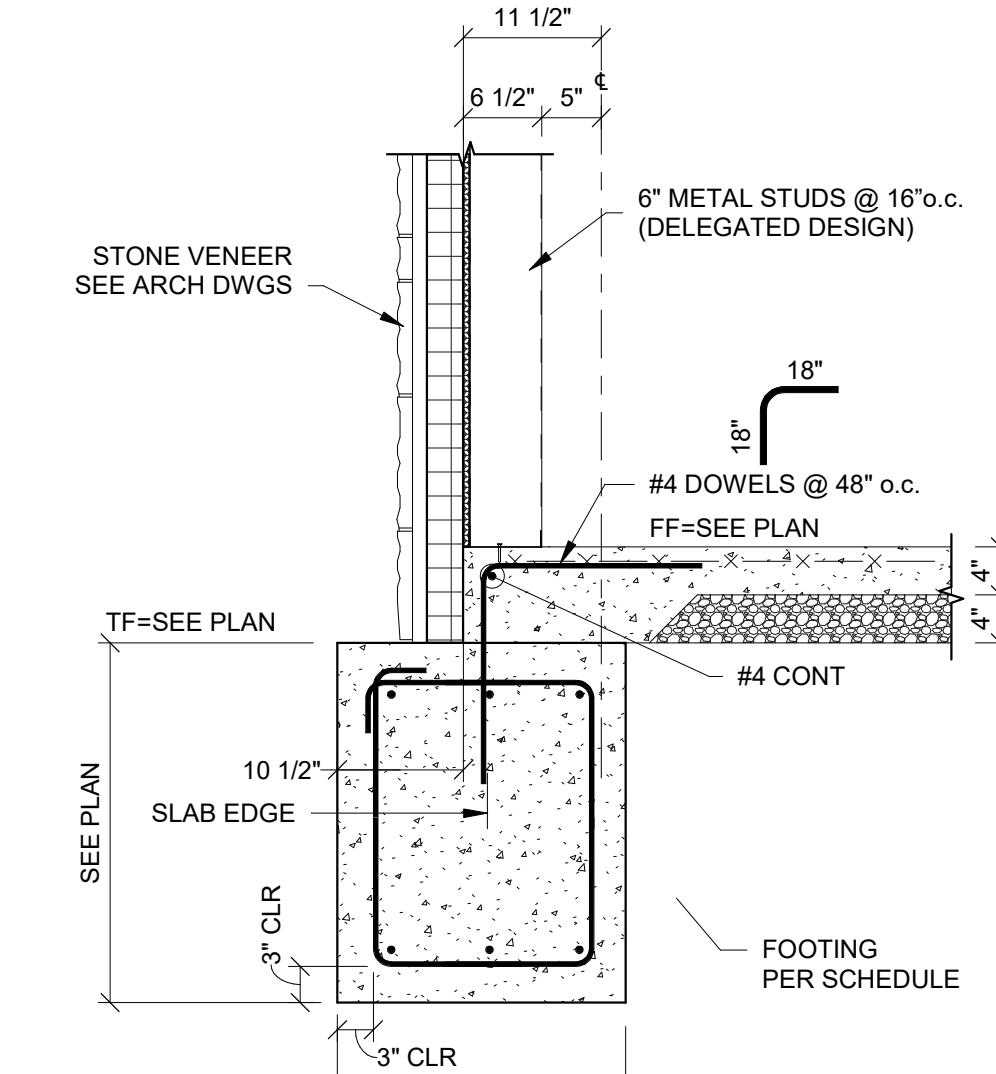
TYPICAL EXTERIOR 6" METAL STUD WALL



TYPICAL EXTERIOR 6" METAL STUD WALL @ DRIVE THRU

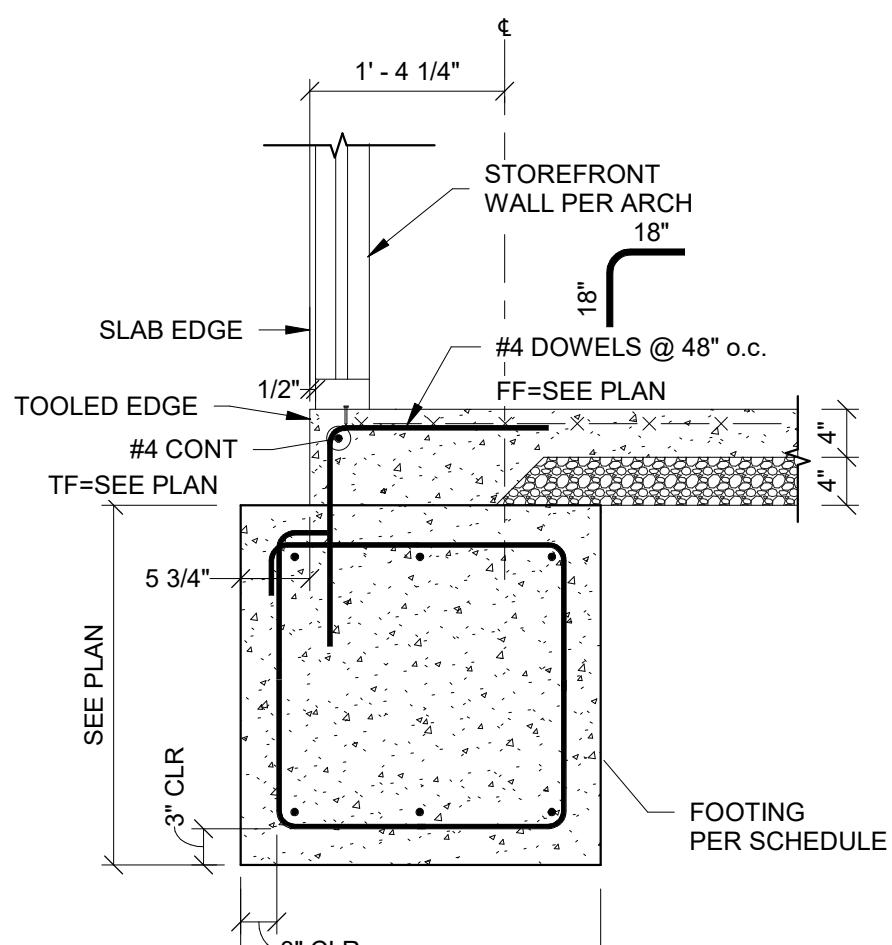


TYPICAL @ WINDOW



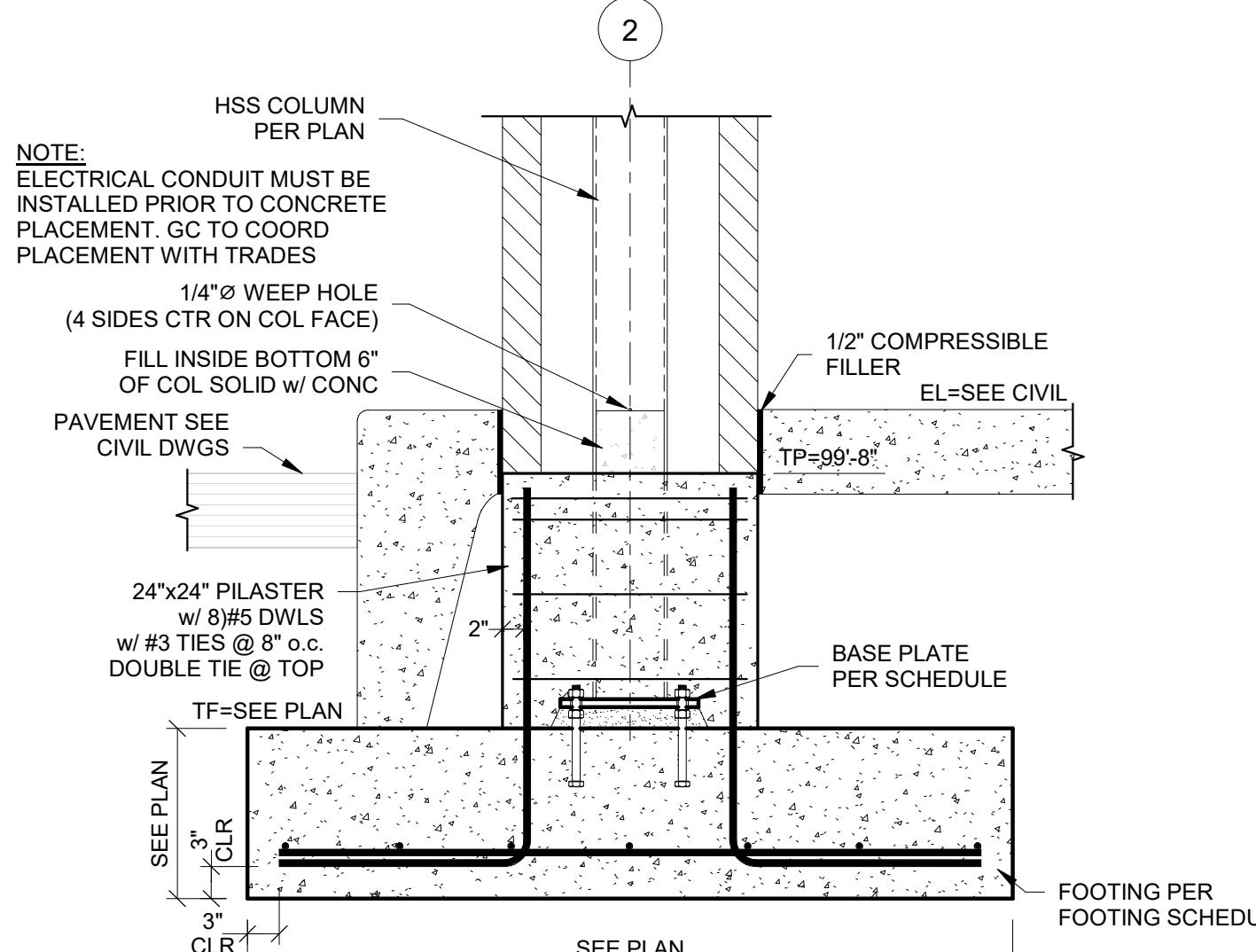
TYPICAL STUD WALL @ STONE

1 SECTION
S2.0 3/4" = 1'-0"



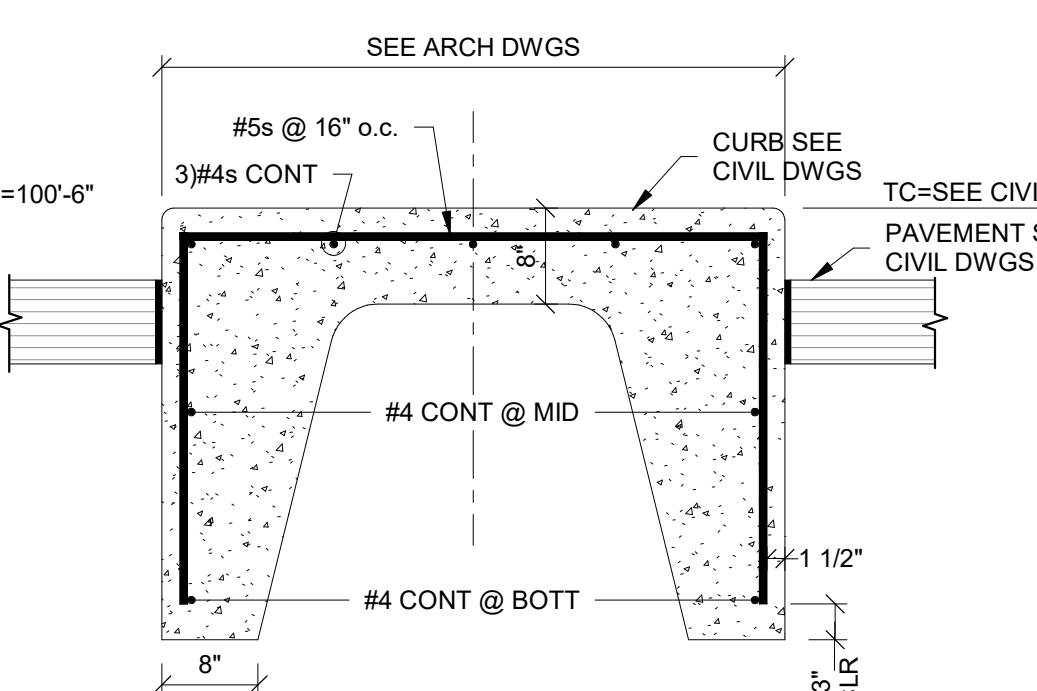
5 SECTION
S2.0 3/4" = 1'-0"

2 SECTION
S2.0 3/4" = 1'-0"

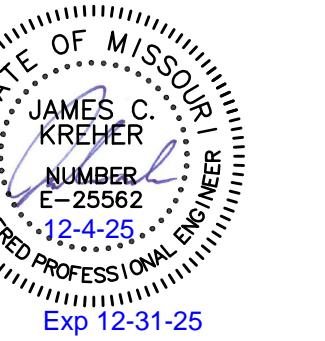


6 SECTION
S2.0 3/4" = 1'-0"

3 SECTION
S2.0 3/4" = 1'-0"



7 SECTION
S2.0 3/4" = 1'-0"



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sions:

ROOF FRAMING SECTIONS

S4.0

ISSUE DATE: 12.04.2025

JOB NUMBER: 25-021

